

James Bay Lithium Pegmatite Project – Potential Borrow Source Assessment

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#### **Table of Contents**

<b>1.0</b> 1.1		ON	
1.1	-	TERIAL REQUIREMENTS	
1.2		AND BACKGROUND REVIEW	
2.0			<b>•</b> • •
<b>2.0</b>		OGY TO INTERPRETATION AND IDENTIFICATION OF POTENTIAL	2.3
2		URCES	24
2.2		TIGATION	
2.3	-	Y TESTING	
2.4	ANALYSIS OF	SUITABLE MATERIAL VOLUME CALCULATION	2.5
3.0	RESULTS OF	THE BORROW INVESTIGATION	3.6
3.1	IDENTIFICAT	ION OF POTENTIAL BORROW SOURCES	3.6
3.2	DISTANCES /	AND ACCESS ROADS	3.7
3.3	FIELD INVES	TIGATION AND LABORATORY TESTING RESULTS	3.7
3.4	VOLUME CAL	CULATION	3.9
4.0	CONCLUSIO	N AND RECOMMENDATIONS	4.9
5.0	REFERENCE	S	5.1
LIST	OF TABLES		
Table '	1: BORROW MA	TERIAL REQUIREMENTS FOR THE YEAR 0 OF THE MINE LIFE	
Table 2	2 SUMMARY TA	BLE	
Table 3	<b>3</b> GRAIN SIZE A	NALYSUS RESULTS FOR SAMPLES COLLECTED IN POTENTIAL SOUR	CES
LIST		ES	
APPE	NDIX A	STATEMENT OF GENERAL CONDITIONS	
APPE	NDIX B	MAPS 1 AND 2 (2-1 À 2-4)	
		POTENTIAL BORROW SOURCE SUMMARY TABLE	

- **APPENDIX D** TEST PIT REPORTS
- **APPENDIX E** LABORATORY TESTING REPORTS
- APPENDIX F PHOTOGRAPHIC ALBUM



## **1.0 INTRODUCTION**

#### 1.1 GENERAL

Stantec Consulting Ltd. (Stantec) was contracted by Galaxy Lithium (Canada) Inc. to complete a potential borrow source assessment in the vicinity of the projected James Bay Lithium project, located in the surroundings of the road stop KM 381 along the James Bay Road. Stantec's scope of work and related deliverables were authorized based on PCN6 variance notice dated from July 15<sup>th</sup>, 2019.

The main objective of the borrow source assessment was to identify potential granular borrow materials for the future needs of the James Bay Lithium project. The scope of work included the following task:

- Review of aerial photos provided by the client;
- Review of WSP's April 2018 report (*Detailed map of surface deposits and identification of potential borrow sources*) and existing surficial geology maps and reports;
- Review of quantities of borrow materials (sand and gravel, till) required for the project (BOQs);
- Based on the review of the existing information described above, identify the potential borrow sources that offer the best potential to meet the project needs;
- Proceed with a preliminary sampling of the identified potential borrow sources with the best available equipment/method that do not involve tree clearance;
- Conduct limited laboratory testing to characterize the material and to determine the suitability for the project needs;
- Issue a report presenting the results of the review and of the preliminary characterization of the materials from the selected borrow sources, including recommendations regarding the second phase of borrow source assessment.

The results of this study will produce a preliminary assessment that will require further site investigation and laboratory testing to refine this assessment.



#### **1.2 BORROW MATERIAL REQUIREMENTS**

The borrow material requirements are not yet known for the entire mine life. However, the following quantities presented in table 1 are anticipated to be needed for the construction of the mine site (year 0).

Borrow material type	Volume (m³)
Compacted till or clay	531,895
Gravel, clear	77,730
Gravel/ crushed rock – 31.5 mm minus	32,760
Gravel/ crushed rock – 75 mm minus	57,150
Riprap D 50 – 100 mm	13,300
Sand and gravel	456,000
Total:	1,168,835

Table 1: Borrow material requirements for the year 0 of the mine life

It may be noted that borrow material produced using rock from quarry sites, including clear gravel, crushed rock and riprap, are excluded from this assessment. The emphasis was on the identification of most favorable sites for the extraction of sand and gravel, sand and till.

Borrow material resource estimate studies are often described as being prospective, probable or proven (GNWT, 2015).

- A <u>prospective</u> volume is one where existence, extent, thickness and quality are inferred on the basis of limited direct evidence such as aerial photo interpretation or satellite imaging but without sampling or ground-truth reconnaissance.
- A <u>probable</u> volume is one where extent, thickness and quality are inferred on the basis of direct and indirect evidence such as aerial photo interpretation, geophysical data, terrain analysis and limited sampling.
- A <u>proven</u> volume is one where extent, thickness, and quality is supported by ground-truth information such as intrusive exploration (test pitting, drilling, etc.), exposed stratigraphic sections and sampling.

The current assessment corresponds to both prospective and probable levels as aerial photo interpretation, desktop terrain analysis and limited field sampling were conducted on most sand and sand and gravel deposits. However, proposed till deposits were not field investigated as part of this assessment as the material type founded in these deposits (gravels, cobbles and boulders) is too coarse for manual (hand shovel/ hand auger) field reconnaissance.



#### 1.3 STUDY AREA AND BACKGROUND REVIEW

The study area occupies approximately 23 km long by 10 km width which is roughly centered on the James Bay Road, between the Amiskv Matawaw Lake (369+700) and the Eastmain River (394+000). The study area was enlarged from the initial aerial photo coverage provided by the client, mainly southward, in order to include areas were numerous deposits appeared to present suitable borrow materials. The total targeted land covers an area of approximately 23,000 ha (Figure 1; Appendix B).

The study area is generally characterized by flat to gently undulating terrain. The elevation ranges from approximately 200 m above sea level, in the northern portion of the study area, to approximately 260 m above sea level, on a hilltop in the southern and southeastern portion of the study area. The highest elevation terrains consist mainly of bedrock outcrops, fluted till and beach ridges while most of the low land is occupied by wetlands.

Regional bedrock geology mapping shows that the study area is underlain by Archean bedrock belonging to the Superior Province, and covering two sub-provinces: the "La Grande sub-province" in the north, and the "Nemiscau sub-province" in the south. The area mainly comprises metasedimentary rocks such as paragneiss and schist, as well as mafic and intermediate volcanic rocks such as basalt, andesite and some volcanoclastic rocks. Igneous granite and granodiorite intrusions are also found (SIGEOM 2019).

During the Late Wisconsin Glaciation (24,000 to 8,000 years before present (BP)), the James Bay region was covered by the Labrador ice sheet. During this glaciation, large amounts of materials were transported and subsequently deposited as till (morainal deposits) across the landscape. Following the ice melt, the marine transgression of the Tyrell sea occurred around 7,900 BP (Hardy 1977). Glaciomarine silt and clay accumulated in the low-lying areas and coarser deposits accumulated along the former Tyrell sea shorelines. Peat bogs and fens have accumulated over the glacial and non-glacial deposits, especially over poorly drained glaciomarine and morainal (till) deposits.

### 2.0 METHODOLOGY

The main objective of the borrow source assessment was to identify areas where suitable and sufficient granular materials are present. The assessment consisted of the following three (3) main steps:

- 1. Aerial photo interpretation and identification of potential borrow sources;
- 2. Field investigation of pre-identified potential borrow sources in order to acquire preliminary ground-truth information such as the texture and the thickness of suitable materials;
- 3. Laboratory testing and granular volume estimation.



#### 2.1 AERIAL PHOTO INTERPRETATION AND IDENTIFICATION OF POTENTIAL BORROW SOURCES

The desktop aerial photo interpretation was completed previously to the field investigation in July 2019. The aerial photo interpretation was conducted using soft copy stereo-models (20 cm accuracy) (1737QBEC0006\_001 to 1737QBEC0006\_059) which were visualized with ESRI ArcGIS® and Purview® softwares. The aerial photo interpretation exercise was completed using a mirror stereoscope for the 3D visualization of black and white, 1: 60,000 scale, on hard copy aerial photos taken in September 1987 (A27202\_2 to A27202\_6). Theses aerial photos were used in order to assess a high potential sector, previous identified using Google Earth, that was not included within the initial aerial photo coverage. Hard copy aerial photos were purchased from Natural Resources Canada - National air photo library.

These tools allowed the terrain scientist to assess the study area in three-dimensional environment and to identify landforms which appear favorable to present granular materials. In the geomorphological context of the study area, favorable landforms for borrow material extraction usually consist of littoral deposits of the postglacial Tyrell sea since traditional glaciofluvial landforms such as perched delta, esker or outwash deposit are absent west of the Sakami moraine. The absence of these types of landform is due to the presence of a floating ice margin on the James Bay east coast.

The spatial delineation of the potential borrow sources (PBS) is based on the mapper knowledge of Quaternary deposits and on his ability to identify the landforms that have the potential to provide suitable borrow materials. Identified potential borrow sources located within the soft copy aerial coverage were delineated directly in digital format while potential borrow sources located outside of this coverage was drawn directly onto the aerial photos and copied in digital format using ArcGIS software.

Prior to the field investigation, test pit locations were determined based on landform topography and areas with a high potential to contain suitable materials. Spatial distribution of the test pits is important as the material texture may significantly change within the same landform depending of the sea currents, dominant winds and exposition during sediment deposition.

#### 2.2 FIELD INVESTIGATION

A five-day field investigation was conducted in order to acquire preliminary information on material types and thickness of the delineated potential borrow sources. The field investigation was conducted between July 31<sup>st</sup> and August 4, 2019. The field crew consisted of a geomorphologist and a field technician from Stantec.

A total of 26 test pits representing 17 PBS (up to 4 test pits per PBS) was conducted using hand shovels, a pickaxe and a hand auger. It was not possible to use heavy equipment at this stage of the project as no wood clearing permit was yet obtained. Test pit locations were accessed by truck using the James Bay Road and old access roads leading to existing borrow pits, and then, on foot. Only the sites showing good sand and gravel aggregate potential were targeted. Due to technical limitations associated with hand shoveling, the sites identified as potential till borrow sources were not investigated.



#### JAMES BAY LITHIUM PEGMATITE PROJECT - POTENTIAL BORROW SOURCE ASSESSMENT

The depth of the test pits ranged between 0.35 and 3.3 m which is not sufficient to estimate a proven volume. However, the information collected in this phase allow to evaluate the suitability of the material and determine targets for the detailed field investigation that will be conducted subsequently. Material texture and grain size assessments were conducted visually. Some samples were collected for subsequent laboratory analysis. Recording of the texture and grain size information was based on the following terminology:

Qualitative terminology (grain size)

Clay	< 0.002 mm
Silt	0.002 – 0.08 mm
Sand	0.08 – 5 mm
Gravel	5 – 80 mm
Cobbles	80 – 200 mm
Boulders	> 200 mm
Quantitative terminology (proportion)	
Quantitative terminology (proportion) Traces	< 10 %
	< 10 % 10 – 20 %
Traces	
Traces Some	10 – 20 %

Detailed descriptions of the test pits are presented in Appendix D. Digital photos of each test pit or excavated material pile are presented in Appendix F.

#### 2.3 LABORATORY TESTING

All soil samples returned to the laboratory were subjected to detailed visual examination and classification (ASTM D2488 - 17e1, 2017) by a geologist. Representative samples of different types of encountered materials were selected for grain size analysis (sieves).

A total of thirteen (13) samples were analyzed. Their proportion of gravel, sand and fine particles (silt and clay combined) are summarized in Table 3 while detailed laboratory results are presented in Appendix E.

#### 2.4 ANALYSIS OF SUITABLE MATERIAL VOLUME CALCULATION

The volume estimates were calculated by multiplying the borrow source area by the estimated average thickness of suitable materials. The average thickness is based on field investigation results (test pits) and on the photo interpretation of suitable materials in regard to the landform geometry.

Estimated volumes of potential borrow sources are "probable" or "prospective" depending on the availability of ground-truth information. Thus, as field investigation was limited to the sand and sand and gravel potential borrow sources, these represent "probable" volumes while estimated volumes for till potential borrow sources consist of "prospective" volumes.



## 3.0 **RESULTS OF THE BORROW SOURCE ASSESSMENT**

This potential borrow source assessment was conducted following the desktop study conducted by WSP in 2018 (Detailed Map of Surface Deposits and Identification of Potential Borrow Sources – James Bay Lithium Mine). The WSP investigation did not identify any sand and gravel deposits (only sandy borrow sources) while an initial volume of 456,000 m<sup>3</sup> of sand and gravel is required by the client for the year 0 of the life of mine. Based on professional experience, available borehole reports and field observations, some discordances were noted between the interpretation of the surficial materials by WSP and ground-truth data.

In this context, Stantec conducted an additional desktop borrow source assessment on an enlarged study area combined with a preliminary field investigation. The deposits numbering from the WSP report was not retained in order to avoid confusion with the material type and the delineation presented in this report.

#### 3.1 IDENTIFICATION OF POTENTIAL BORROW SOURCES

Using the desktop aerial photo interpretation assessment, 24 deposits (also referred as potential borrow sources) were identified as being more likely to contain suitable granular borrow materials. These deposits consist of littoral and perched beach ridges landforms, which are the most common source of sand and gravel material within the James Bay region, and well as some till ridges. The general location of these potential borrow sources is shown on Figure 1 (Appendix B) while closer views of the potential borrow sources are shown on Figure 2-1 to 2-4 (Appendix B). The main characteristics of the potential borrow sources are summarized in Table 2 (Appendix C).

The material type was at first determined by aerial photo-interpretation and subsequently confirmed by conducting field surveys for most of the sand and gravel deposits. PBS-01 to PBS-13, PBS-15 to PBS-18 and PBS-22 consist of deposits of variable proportions of sand and gravel (including cobbles and boulders). PBS-19, PBS-21 and PBS-23 are mainly composed of sand while PBS-14, PBS-20 and PBS-24 are expected to be till deposits.

The sand and gravel potential borrow source areas range from 1.1 up to 44.5 ha and are surrounded by low-lying wetlands, till deposits and few bedrock outcrops. The average thickness of these deposits is expected to range from 1 to 3 m. Well-defined beach ridges such as PBS-2, PBS-3, PBS-8, PBS-9 and PBS-11 usually show coarser material and thicker suitable materials than gently sloping littoral deposits.

The sand potential borrow sources occupy areas ranging from 11.7 to 45.6 ha and seem to reach several meters thick. PBS-19 and PBS-21 are mostly surrounded by wetlands while PBS-23 is sited on bedrock on its north, east and west boundaries and lowers toward a wetland on its south boundary.

Till deposits consist of materials deposited directly by the glacier ice, with little or no transportation by water. They are usually unstratified and consist of a heterogeneous mixture of clay, silt, sand, gravel and boulders. The selected till deposits consist of gentle to moderate sloping hills where the average elevation range from approximately 2 to 10 m higher than the surrounding terrains. Their orientations generally present a

northeast - southwest trend. They cover areas between 8.1 to 115.5 ha while their average thickness above the groundwater table level is expected to reach approximately 2.5 m.

#### 3.2 DISTANCES AND ACCESS ROADS

In order to compare the potential of each borrow source, preliminary access roads were drawn to evaluate their distance to the James Bay Road. In this context, the proposed access roads do not reflect the potential of a deposit and should not be interpreted as a specific site selection. The Table 2 (Appendix C) presents the total length of the proposed access road as well as the length of the existing roads, if any. It should be noted that some access roads leading to existing pits could be used to reach some of the potential borrow sources. However, the conditions of these existing roads are unknown and should be validated on the field.

The potential borrow source locations range from directly adjacent to the James Bay Road to a distance of approximately 6.8 km. The longest access road to be built would be located at the southern boundary of the study area (PBS-1 to PBS-5). In this area, a large wetland of more than a kilometer should be crossed between the existing access road and the potential borrow sources to avoid a longer road which would involve several water crossing.

Another access constraint is located along the existing access road to PBS-14. At a distance of 1.4 km from the James Bay Road, the existing access road leading to the power line crosses a watercourse of approximately 10 m in width, where no bridge or other crossing structure is present. The construction of a water crossing structure will have to be foreseen if the PBS-14 is retained as a borrow source.

Otherwise, the access to the remaining potential borrow sources only involves crossing of relatively small wetland sections and minor water crossing, with no major constraints. Details characteristics of access roads will be provided for the retained sites at the phase 2 of the potential borrow source assessment.

#### 3.3 FIELD INVESTIGATION AND LABORATORY TESTING RESULTS

During the present field investigation, 26 test pits were opened within the boundaries of 16 potential borrow sources (maps 2-1 to 2-4; Appendix B).

Typically, the encountered local stratigraphy of <u>sand and gravel deposits</u> consists of less than 0.1 m of topsoil which is mainly composed of organic matter, overlying brown to reddish-brown gravelly sand to gravel and sand with traces of silt and clay. The grain size analysis of encountered materials shows between 24.2 to 71.4 % of gravel, 28.3 to 75.3 % of sand and between 0.3 and 8.7 % of fine particles (silt and clay combined). The presence of cobbles and boulders is also common. No unfavorable material was found in most of the test pits, except for PBS-03-1 and PBS-16-1 where a possible contact with the till layer was reached at a depth of 1.7 m and 1.4 m, respectively. The water table was observed only in test pit PBS-11-1 at a depth of 1.2 m. Otherwise, the water table was not reached in the potential sand and gravel deposits.

The stratigraphy of <u>sand deposits</u> (PBS-19, PBS-21 and PBS-23) consists mainly of a thin layer of topsoil, usually less than 5 cm thick, overlying a matrix of sand with variable proportions of gravel and silt. The sand content ranges between 80.0 and 97.7 % for the selected samples. PBS-19-1 and PBS-21-1 present respectively 1.9 and 2.1 % of gravel while PBS-23-1 is coarser showing a proportion of 14.2 % of gravel. A

negligible fraction of fine particles was found in samples PBS-19-1 and PBS-23-1 while the sample PBS-21-1 presents a combine proportion of silt and clay of 17.9%. This proportion of fine particles could be an issue for the utilization of materials from PBS-21 for different kinds of civil engineering work. Cobbles and boulders may be present, but they are less frequent than in the sand and gravel deposits. Unfavorable materials seem to be found in the test pit PBS-21 at a depth of 3.25 m. Below this depth, materials become silty. The water table was not reached in any of the test pits conducted with a sand deposit.

The proportion of gravel, sand and fine particles of representative selected samples is summarized in Table 3. The detailed description of the 26 test pits is presented in Appendix D while the grain size analysis results are provided in Appendix E. A photographic album of each test pit is also presented in Appendix F.

Test pit ID	Material type	Depth (m)	Gravel (%)	Sand (%)	Silt and clay (%)
PBS-02-1	Sand and gravel	0.7 – 1.2	40.9	58.7	0.4
PBS-03-2	Sand and gravel	1.0 – 1.2	71.4	28.3	0.3
PBS-04-1	Sand and gravel	0.55 – 1.2	24.2	75.3	0.4
PBS-08-1	Sand and gravel	0.05 – 1.0	51.7	46.6	1.7
PBS-09-2	Sand and gravel	0.06 – 1.1	65.1	34.0	0.9
PBS-10-1	Sand and gravel	0.0 – 0.6	40.1	57.8	2.1
PBS-11-2	Sand and gravel	0.0 – 1.0	52.2	46.8	1.0
PBS-15-1	Sand and gravel	0.1 – 0.55	45.8	45.5	8.7
PBS-17-1	Sand and gravel	0.5 – 1.1	51.1	47.5	1.4
PBS-22-1	Sand and gravel	0.4 – 0.7	49.1	47.4	3.5
PBS-19-1	Sand	0.55 – 1.4	1.9	97.7	0.4
PBS-21-1	Sand	0.05 – 1.75	2.1	80.0	17.9
PBS-23-1	Sand	0.0 – 0.3	14.2	84.8	1.0

 Table 3 Grain Size Analysis Results for Samples Collected in Potential Borrow Sources.



#### JAMES BAY LITHIUM PEGMATITE PROJECT - POTENTIAL BORROW SOURCE ASSESSMENT

No field investigation was conducted within the boundaries of PBS-5, PBS-6 and PBS12 during this preliminary investigation, as they were deemed to offer a lower potential volume of granular. However, materials within the PBS-5 are anticipated to be similar to those found in PBS-4. The PBS-6 is not anticipated to contain significant volume of suitable material for the construction of the mine site but could be used for the access road leading to PBS-1, PBS-2, PBS-3, PBS-4 and PBS-5. Finally, the potential borrow source PBS-12 is anticipated to present similar materials to PBS-13 as both deposits show a comparable characteristic in terms of location, elevation and landforms

In addition, PBS-14, PBS-20, PBS-24 and PBS-25 were not field investigated because they are expected to be till deposits which are too coarse and dense to be explored using manual tools (hand shovel/ hand auger). However, the existing boreholes drilled as part of geotechnical studies (WSP, 2018; Stantec, 2019) usually described the regional till as a silty sand and gravel with traces of clay to a gravelly sand and silt with traces of clay.

#### 3.4 VOLUME CALCULATION

Based on aerial photo interpretation and the results of the preliminary field investigation, probable and prospective volumes were estimated for the different potential borrow sources. It should be noted, even for the "probable" volume estimation, that the field data collected in August 2019 remain fragmentary and further investigations will be needed to confirm the quality and quantity of suitable materials.

The volume estimations are summarized in Table 2 (Appendix C).

The estimated volumes of sand and gravel potential borrow sources range from 10,000 up to > 300,000 m<sup>3</sup>. The total prospective and potential volume of sand and gravel expected to be found within the study totalize more than 1,407,000 m<sup>3</sup>. The deposits that appear to present the best potential are grouped in two sectors: the first sector included PBS-1 to PBS-4 while the second sector included PBS-8, PBS-9 and PBS-11. Sandy materials were not requested for the year 0 of the mine life. However, three (3) substantial potential borrow sources of sandy materials were identified on the west side of the James Bay Road for future needs. The deposits PBS-19 and PBS-23 are expected to contain > 500,000 m<sup>3</sup> and > 200,000 m<sup>3</sup>, respectively. The materials found in the potential borrow source PBS-21 could not be appropriated for civil engineering work as they present a high proportion of fine particles (17.9 %). Finally, the three (3) identified potential borrow sources consisting of till present significant volumes. The largest deposit, PBS-14, is expected to contain more than a million cubic meters of materials while PBS-20 and PBS-24 are expected to contain > 250,000 and > 150,000 m<sup>3</sup>.

### 4.0 CONCLUSION AND RECOMMENDATIONS

This potential borrow source assessment was conducted using aerial photo interpretation followed by a five-day field reconnaissance. The initial study area was determined by the aerial photo coverage provided by the client to which a southern extension was added in order to include an area of expected suitable materials. The final study area extends between the Amiskv Matawaw Lake and the Eastmain River which

correspond approximately to KP 369+700 to 394+000 of the James Bay Road. The information gathered during this assessment is assumed to represent a probable and a prospective level of investigation.

Twenty-four (24) potential borrow sources were identified during the desktop aggregate assessment. Eighteen (18) of them contain sand and gravel, three (3) are composed of sand and three (3) consist of till deposits. A total of 26 test pits were advanced to depths ranging from 0.35 to 3.3 m in the potential borrow sources composed of sand and gravel and sand. Till deposits were not part of this field investigation as their materials are too coarse and dense to be excavated using manual tools. The utilization of mechanical equipment was not an option at this stage as the tree clearing permit was yet to be obtained.

Materials encountered in potential borrow sources PBS-1 to PBS-13, PBS-15 to PBS-18 and PBS-22 consist mainly of gravelly sand to sandy gravel with traces of silt and clay. The presence of cobbles and boulders are also common within these deposits. Potential borrow sources PBS-19, PBS-21 and PBS-23 are mainly composed of sand matrix with low proportion of gravel and silt. However, a sample collected within the potential borrow source PBS-21 shows a high proportion of fine particles (17.9 %) indicating that the deposit should not be used for civil engineering work. The potential borrow sources PBS-14, PBS-20 and PBS-24 are expected to consist of till.

Grain size analysis conducted on 13 representative samples showed that the proportion of fine particles (combined proportion of silt and clay) are in most case less than 10%, and more often less than 5%. The sample collected within the potential borrow source PBS-21 (sample PBS-21-1) is the only one to exceed the proportion of 10% of fine particles showing a combined fraction of silt and clay of 17.9 %.

This initial potential borrow source investigation should be completed by a detailed field investigation program to confirm the composition and quality of selected deposits, the thickness variation of the suitable materials and well as the water table depth. This last parameter has a direct influence on the potential extractable volume as a pit cannot be mined deeper than a meter above the water table level. This second phase of field investigation should be conducted using a hydraulic shovel. While most of the deposits should be relatively easy to access, the sector gathering PBS-1 to PBS-5 could be challenging to reach using heavy equipment as wetlands and/or water crossing will have to be crossed.

Based on the expected materials quality and thickness as well as the potential borrow material volume the second phase of investigation should focus on potential borrow sources PBS-2, PBS-3, PBS-4, PBS-8, PBS-9 and PBS-11 as they are anticipated to present a high potential for sand and gravel material. Despite sandy material are not required at this point, further investigations should be conducted in PBS-23 as a pit have already been mined in the same deposit and it's easily accessible. Finally, till deposits PBS-14, PBS-20 and PBS-24 should be sampled during the second phase of the investigation. The deposits PBS-20 and PBS-24 are easily accessible but the extractable granular volume could be limited because of the local underground water conditions, as for PBS-14, this borrow could provide a significant till volume.

Finally, it must be noted that the location of the borrow pits and the extraction of materials are submitted to the Chapter Q-2 r.7 of the Environment Quality Act (EQA). The final selection of borrow sources should be done according the applicable regulations.



#### 5.0 **REFERENCES**

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# **APPENDIX A**

**Statement of General Conditions** 

#### STATEMENT OF GENERAL CONDITIONS

<u>USE OF THIS REPORT</u>: This report has been prepared for the sole benefit of the Client or its agent and may not be used by any third party without the express written consent of Stantec Experts-conseils and the Client. Any use which a third party makes of this report is the responsibility of such third party.

<u>BASIS OF THE REPORT</u>: The information, opinions, and/or recommendations made in this report are in accordance with Stantec Experts-conseils present understanding of the site specific project as described by the Client. The applicability of these is restricted to the site conditions encountered at the time of the investigation or study. If the proposed site specific project differs or is modified from what is described in this report or if the site conditions are altered, this report is no longer valid unless Stantec Experts-conseils is requested by the Client to review and revise the report to reflect the differing or modified project specifics and/or the altered site conditions.

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INTERPRETATION OF SITE CONDITIONS: Soil, rock, or other material descriptions, and statements regarding their condition, made in this report are based on site conditions encountered by Stantec Experts-conseils at the time of the work and at the specific testing and/or sampling locations. Classifications and statements of condition have been made in accordance with normally accepted practices which are judgmental in nature; no specific description should be considered exact, but rather reflective of the anticipated material behavior. Extrapolation of in situ conditions can only be made to some limited extent beyond the sampling or test points. The extent depends on variability of the soil, rock and groundwater conditions as influenced by geological processes, construction activity, and site use.

<u>VARYING OR UNEXPECTED CONDITIONS</u>: Should any site or subsurface conditions be encountered that are different from those described in this report or encountered at the test locations, Stantec Experts-conseils must be notified immediately to assess if the varying or unexpected conditions are substantial and if reassessments of the report conclusions or recommendations are required. Stantec Experts-conseils will not be responsible to any party for damages incurred as a result of failing to notify Stantec Experts-conseils that differing site or sub-surface conditions are present upon becoming aware of such conditions.

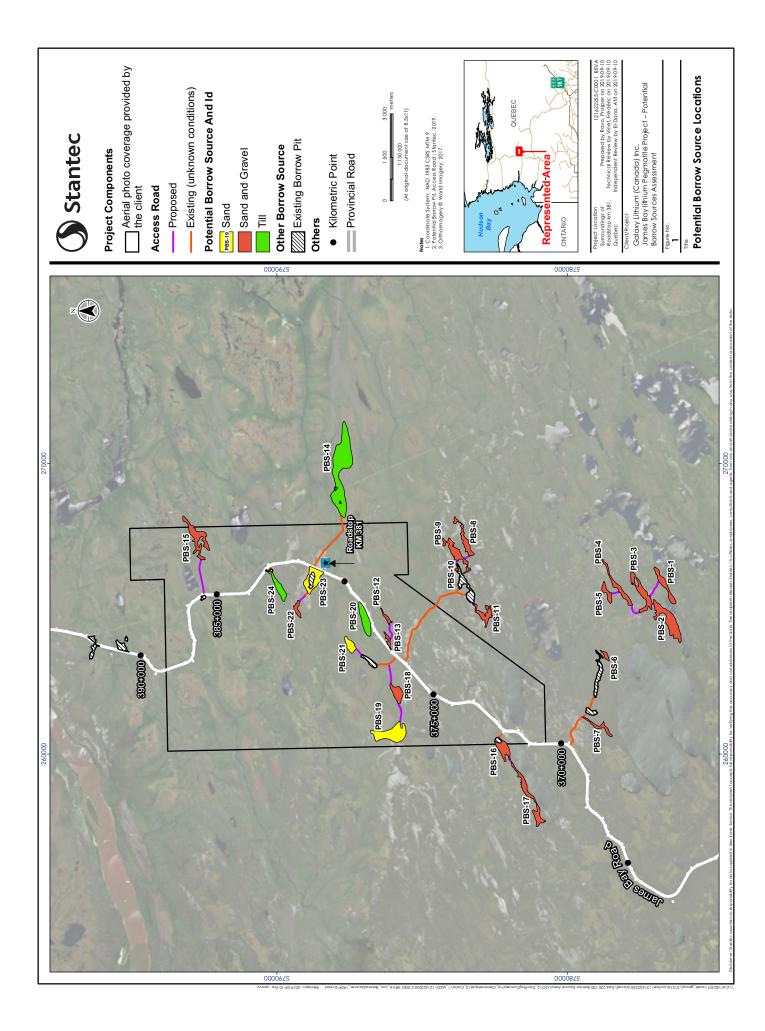
<u>PLANNING, DESIGN, OR CONSTRUCTION</u>: Development or design plans and specifications should be reviewed by Stantec Experts-conseils, sufficiently ahead of initiating the next project stage (property acquisition, tender, construction, etc), to confirm that this report completely addresses the elaborated project specifics and that the contents of this report have been properly interpreted. Specialty quality assurance services (field observations and testing) during construction are a necessary part of the evaluation of sub-subsurface conditions and site preparation works. Site work relating to the recommendations included in this report should only be carried out in the presence of a qualified geotechnical engineer; Stantec Experts-conseils cannot be responsible for site work carried out without being present.

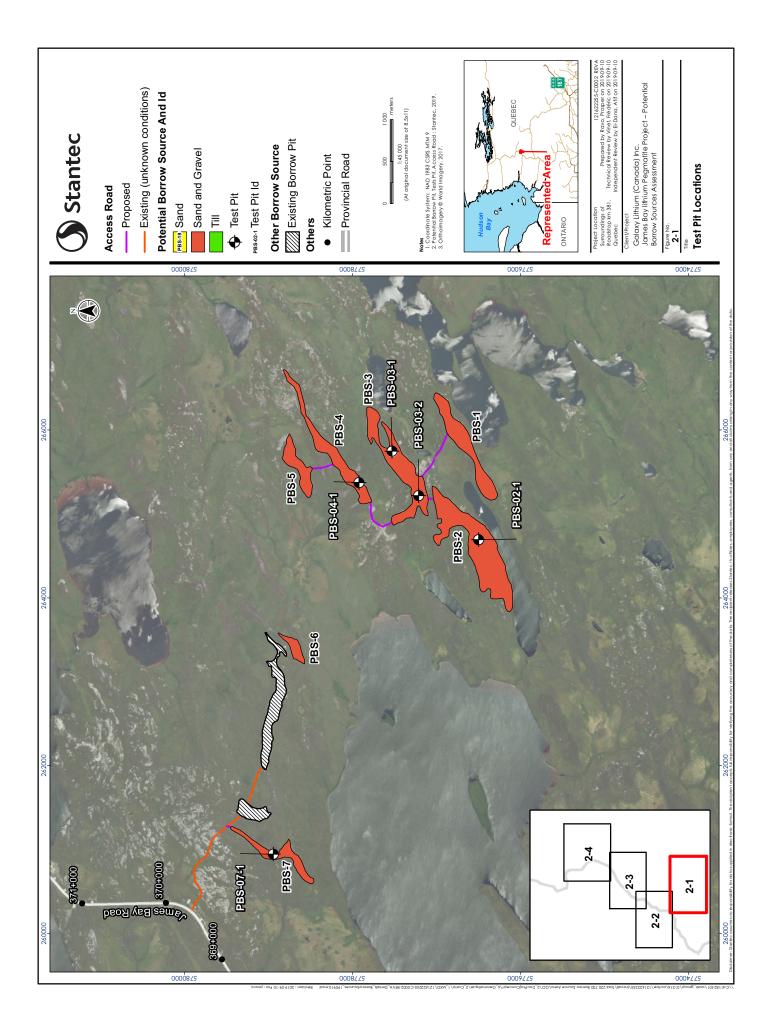
Stantec

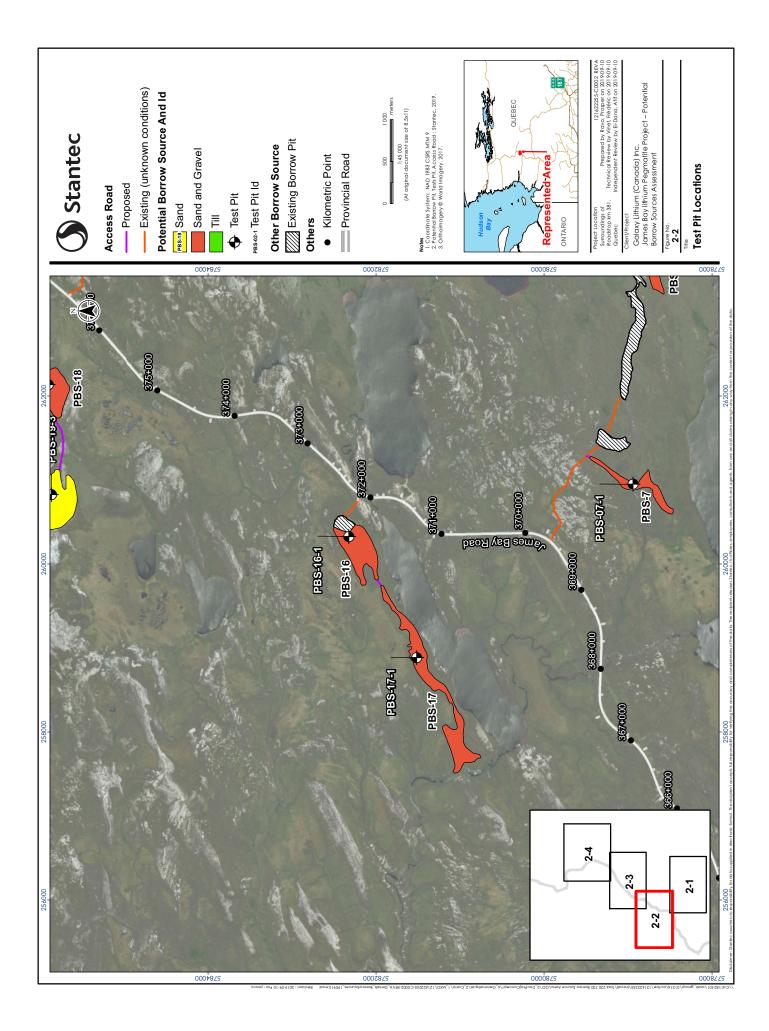
## **APPENDIX B**

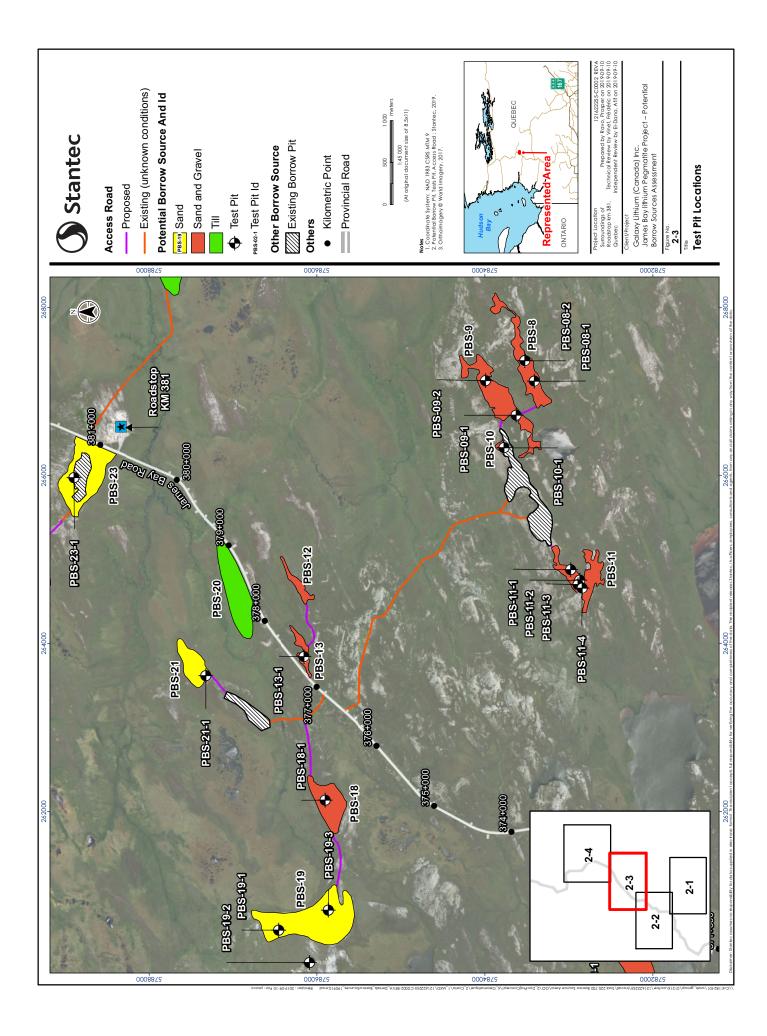
Maps 1 and 2 (2-1 to 2-4)

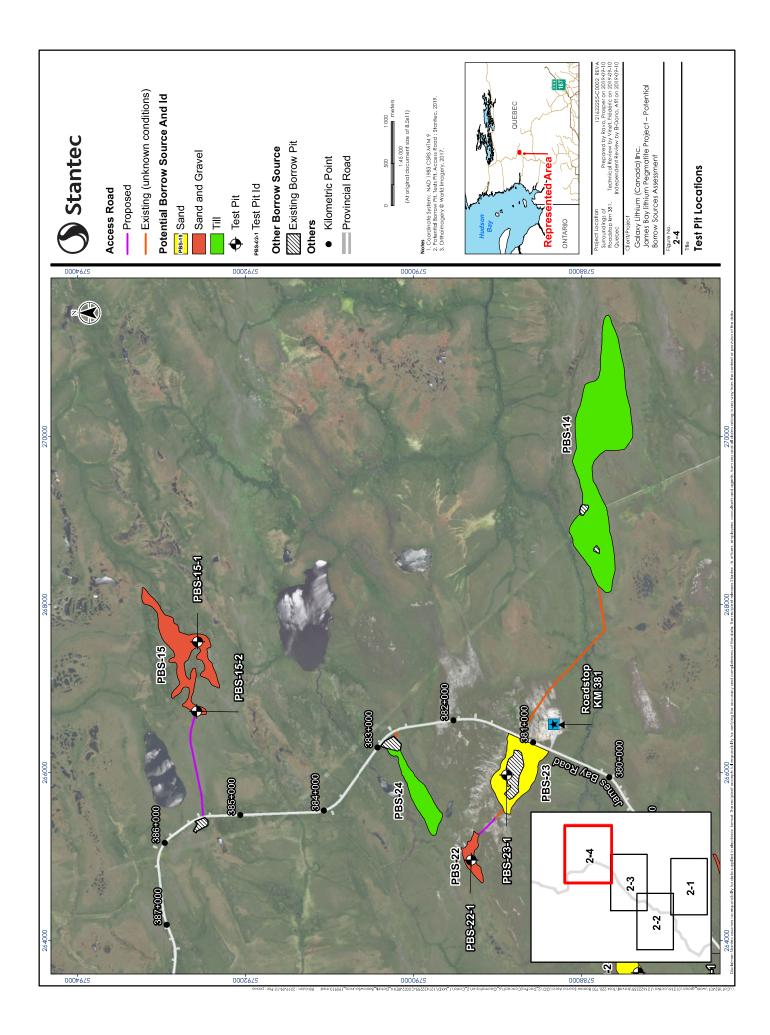












# **APPENDIX C**

## Potential Borrow Source Summary Table



# Table 2: Summary table

Aea (ha) (ha) (19.3 (19.3 (19.3 (19.3 (17.9)	Approximative et distance to reach the James Bay to reach the James Bay to the Road (km) control Road	existing road - unknown         exit vol vol (km)         exit vol sta           (am)         3.6         Pros           3.6         Pros         3.6           4.5         Pros         4.5           4.5         Pro         Pro	Estimated average statusaverage suitable mitable imikknessProspective2.0Probable2.5Probable2.5Probable2.5Probable2.5Probable1.5Probable1.5Probable1.5Probable2.5Probable2.5Probable2.5Probable1.0Probable2.5Probable2.5Probable2.5Probable2.5Probable2.5	Estimated volume volume > 1 volume > 100,000 > 150,000 > 150,000 75,000 12,000 112,000 112,000 112,000 112,000 112,000 112,000 112,000 110,000	Commans         Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties.         Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties.         Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties.         Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties.         Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties.         Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties.         Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties.         Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties.         Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties.         Beach ridge that could be used for access construction.         Beach ridges overlying an northeast/southwest oriented till ridge.         Beach ridges overlying bedrock.         Beach ridges overlying bedrock.         Beach ridges overlying bedrock.         Beach ridges overlying bedrock.
Sand and gravel     19.3       gravel     19.3       gravel     44.5       Sand and gravel     27.7       Sand and gravel     27.7       Sand and gravel     9.7       Sand and gravel     9.7       Sand and gravel     10       Sand and gravel     1.1       Sand and gravel     1.1       Sand and gravel     1.1       Sand and gravel     4.5       Sand and gravel     4.5       Sand and gravel     4.5       Sand and gravel     1.1       Sand and gravel     4.5       Sand and gravel     4.5       Sand and gravel     1.1       Sand and gravel     4.5       Sand and gravel     1.1       Sand and gravel     4.5       Sand and gravel     1.1.5       Sand and gravel     1.1.5       Sand and gravel     1.1.7       Sand and gravel     1.1.7       Sand and gravel     1.1.7       Sand and gravel     1.1.7					Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridge that could be used for access construction. Beach ridge that could be used for access construction. Beach ridge that could be used for access construction. Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 776 791         Sand and gravel         19.           5 776 687         Sand and gravel         44.           5 777 582         Sand and gravel         44.           5 777 582         Sand and gravel         18.           5 778 379         Sand and gravel         18.           5 778 379         Sand and gravel         18.           5 779 105         gravel         10.           5 783 635         Sand and gravel         11.           5 783 945         Sand and gravel         11.           5 783 945         Sand and gravel         4.5           5 783 930         gravel         11.           5 783 930         gravel         11.           5 782 938         Sand and         4.5           5 782 930         gravel         11.           5 782 933         Sand and         4.5           5 782 833         Sand and         11.           5 782 833         Sand and         1.1           5 782 833	6.8 6.2 5.6 5.5 5.5 7 4.2 5 5.5 4.6 4.2 5 5.5 7 4.2 5 5.5 7 4.2 5 5 5 7 4.2 5 5 7 1 3 7 7 1 3 7 1 3 7 6 6 2 1 5 6 5 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6			> 100,000 > 300,000 > 150,000 75,000 30,000 12,000 15,000 15,000 175,000 10,000 10,000	Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficuties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficuties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficuties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficuties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficuties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficuties. Small littoral deposit that could be used for access construction. Beach ridge that could be used for access construction. Beach ridges overlying an northeast/southwest oriented till ridge. Beach ridges overlying an northeast/southwest oriented till ridge. Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 776 687         Sand and gravel         44.           5 777 582         Sand and gravel         27.           5 778 379         Sand and gravel         27.           5 778 379         Sand and gravel         3.           5 778 875         Sand and gravel         3.           5 778 875         Sand and gravel         10           5 779 105         Sand and gravel         10           5 783 635         Sand and gravel         11.           5 783 935         Sand and gravel         11.           5 783 935         Sand and gravel         11.           5 783 930         Sand and gravel         4.5           5 783 930         Sand and gravel         11.           5 782 930         Sand and gravel         4.5           5 782 930         Sand and gravel         16.           5 782 930         Sand and gravel         16.           5 782 930         Sand and gravel         16.           5 782 933         Sand and gravel         11.           5 782 833         Sand and gravel         18.           5 782 833         Sand and gravel         18.           5 782 833         Sand and gravel         18.           5 781 57         S	6.2 5.6 5.5 5.4 5.4 3.7 1.3 1.3 1.3 4.6 4.6 4.25 4.25			<ul> <li>&gt; 300,000</li> <li>&gt; 150,000</li> <li>75,000</li> <li>30,000</li> <li>12,000</li> <li>15,000</li> <li>15,000</li> <li>175,000</li> <li>10,000</li> <li>10,000</li> </ul>	Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridge that could be used for access construction. Beach ridge that could be used for access construction. Beach ridges overlying an northeast/southwest oriented till ridge. Beach ridges overlying an northeast/southwest oriented till ridge. Beach ridges overlying bedrock. Easily accessible using access road. Adjacent to an existing pit.
5 777 582         Sand and gravel         27.           5 778 379         Sand and gravel         18           5 778 379         Sand and gravel         9.7           5 778 775         Sand and gravel         10           5 779 105         gravel         10           5 783 945         gravel         11           5 782 988         gand and         4.5           5 782 830         gravel         11           5 782 831         gravel         18           5 782 833         Sand and         18           5 782 833         gravel         18           5 782 833         gravel         18	5.6 5.5 5.4 5.4 7 3.7 1.3 7.5 5.5 5.5 4.6 4.6 4.25 7.0			<ul> <li>&gt; 150,000</li> <li>75,000</li> <li>30,000</li> <li>12,000</li> <li>15,000</li> <li>175,000</li> <li>&gt; 200,000</li> <li>10,000</li> </ul>	Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Small littoral deposit that could be used for access construction. Beach ridge that could be used for access construction. Beach ridges overlying an northeast/southwest oriented till ridge. Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 778 379         Sand and gravel         18           5 778 379         graval         9.7           5 778 875         Sand and gravel         3.5           5 778 875         Sand and gravel         10           5 778 375         Sand and gravel         17.           5 783 635         Sand and gravel         17.           5 783 935         Sand and gravel         17.           5 783 935         Sand and gravel         17.           5 783 930         Sand and gravel         14.           5 783 930         Sand and gravel         16.           5 782 998         Sand and gravel         4.6           5 782 930         Sand and gravel         115           5 782 930         Sand and gravel         115           5 782 833         Sand and gravel         115           5 782 833         Sand and gravel         18.           5 781 57         Sand and	5.5 5.4 3.7 1.3 5.5 5.5 4.6 4.2 5 0 0			75,000 30,000 12,000 15,000 775,000 > 200,000	Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Small littoral deposit that could be used for access construction. Beach ridge that could be used for access construction. Beach ridges overlying an northeast/southwest oriented till ridge. Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 778 785         Sand and gravel         9.7           5 778 775         Sand and gravel         3.9           5 779 105         Sand and gravel         10           5 779 105         Sand and gravel         11.1           5 783 635         Sand and gravel         11.1           5 783 930         Sand and gravel         11.1           5 783 930         Sand and gravel         11.1           5 783 930         Sand and gravel         11.1           5 782 938         Sand and gravel         4.5           5 782 930         Sand and gravel         11.1           5 786 380         Sand and gravel         11.1           5 782 833         Sand and gravel         13.4           5 782 833         Sand and gravel         18.4           5 782 833         Sand and gravel         28.           5 781 577         Sand and gravel         10.1           5 786 291         Sand and gravel         45.1           5 787 652         Sand and gravel         45.1           5 787 652	5.4 3.7 1.3 5.5 5.5 4.6 4.2 5 70			30,000 12,000 15,000 > 200,000 10,000	Beach ridges overlying an northeast/southwest oriented till ridge. Expected access difficulties. Small littoral deposit that could be used for access construction. Beach ridge that could be used for access construction. Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using access road. Adjacent to an existing pit.
5 778 875         Sand and gravel         3.9           5 779 105         Sand and gravel         10           5 783 635         Sand and gravel         17.1           5 783 945         Sand and gravel         17.1           5 783 930         Sand and gravel         17.1           5 783 930         Sand and gravel         1.1           5 782 998         Sand and gravel         1.1           5 782 998         Sand and gravel         4.6           5 782 930         Sand and gravel         4.6           5 786 346         Sand and gravel         4.6           5 786 346         Sand and gravel         4.6           5 782 833         Sand and gravel         115           5 782 451         gravel         18.           5 782 833         Sand and gravel         18.           5 782 833         Sand and gravel         18.           5 782 833         Sand and gravel         18.           5 782 831         Sand and gravel         16.           5 782 831         Sand and gravel         16.           5 782 831         Sand and gravel         16.           5 782 931         Sand and gravel         16.           5 787 852         San	3.7 1.3 5.5 4.6 4.2 7 0			12,000 15,000 175,000 > 200,000 10,000	Small littoral deposit that could be used for access construction. Beach ridge that could be used for access construction. Beach ridges overlying an northeast/southwest oriented till ridge. Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 779 105         Sand and gravel         17.           5 783 635         Sand and gravel         17.           5 783 945         Sand and gravel         24.           5 783 945         Sand and gravel         1.1           5 783 930         Sand and gravel         1.1           5 782 998         Sand and gravel         4.5           5 782 998         Sand and gravel         4.5           5 786 380         Gravel         16.           5 786 380         Gravel         4.5           5 786 380         Gravel         16.           5 786 380         Gravel         4.6           5 782 933         Sand and gravel         115           5 782 833         Sand and gravel         18.           5 782 833         Sand and gravel         18.           5 782 851         Gravel         18.           5 786 069         Gravel         18.           5 787 652         Sand         45.           5 787 652         <	1.3 5.5 4.6 4.2 7.0			15,000 175,000 > 200,000 10,000	Beach ridge that could be used for access construction. Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using access road. Adjacent to an existing pit. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 783 635         Sand and gravel         17.           5 783 945         Sand and gravel         24.           5 783 930         Sand and gravel         1.1           5 783 930         Sand and gravel         1.1           5 783 930         gravel         16.           5 782 998         Sand and gravel         4.6           5 786 346         gravel         4.6           5 786 346         gravel         4.6           5 787 825         Till         115           5 782 833         gravel         34.           5 782 833         gravel         18.           5 782 850         gravel         18.           5 782 833         gravel         18.           5 782 833         gravel         18.           5 782 831         gravel         16.           5 782 815         Sand and         16.           5 786 939         gravel         16.           5 787 852         Sand and         11.           5 787 852         Sand         45. <td>5.5 4.6 4.2 2.5</td> <td></td> <td></td> <td>175,000 &gt; 200,000 10,000</td> <td>Beach ridges overlying an northeast/southwest oriented till ridge. Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.</td>	5.5 4.6 4.2 2.5			175,000 > 200,000 10,000	Beach ridges overlying an northeast/southwest oriented till ridge. Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 783 945         Sand and gravel         24.           5 783 930         Sand and gravel         1.1           5 782 998         Sand and gravel         16.           5 782 998         Sand and gravel         4.5           5 782 998         Sand and gravel         4.5           5 786 346         Sand and gravel         4.6           5 787 825         Till         115           5 782 833         Sand and gravel         34.           5 782 833         Sand and gravel         18.           5 782 851         Till         115           5 782 833         Sand and gravel         18.           5 782 851         Sand and gravel         18.           5 782 851         Sand and gravel         18.           5 782 851         Sand and gravel         18.           5 786 069         Sand and gravel         16.           5 786 291         Sand and gravel         45.           5 787 652         Sand and gravel         45.           5 787 652         Sand         45.	4.6 4.2 4.25			> 200,000 10,000	Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 783 930         Sand and gravel         1.1           5 782 998         Sand and gravel         16.           5 786 380         gravel         4.5           5 782 351         gravel         4.6           5 782 833         Sand and         4.8           5 792 833         Sand and         1115           5 792 833         Sand and         18.           5 782 451         Sand and         18.           5 781 577         Sand and         18.           5 781 577         Sand and         18.           5 786 069         Sand and         16.           5 786 291         Sand and         16.           5 786 291         Sand and         16.           5 787 123         Till         25.           5 787 652         Sand         45.1           5 787 652         Sand         45.1	4.2 4.25 0.0			10,000	Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit. Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 782 998         Sand and gravel         16.           5 786 380         Sand and gravel         4.5           5 786 346         Sand and gravel         4.6           5 787 825         Till         115           5 782 833         Sand and gravel         4.6           5 782 833         Sand and gravel         34.           5 782 833         Sand and gravel         18.           5 781 577         Sand and gravel         16.           5 786 069         Sand and gravel         16.           5 786 291         Sand and gravel         45.           5 787 652         Sand         11.           5 787 652         Sand         11.	4.25 0.a		2		Beach ridges overlying bedrock. Easily accessible using existing access road. Adjacent to an existing pit.
5 786 380         Sand and gravel         4.5           5 786 346         Sand and gravel         4.6           5 787 825         Till         115           5 782 833         Sand and gravel         34, 94, 94, 94, 94, 94, 94, 94, 94, 94, 9	00	4.2 Pro	Î	75,000	
5 786 346         Sand and gravel         4.6           5 787 825         Till         115           5 792 833         Sand and gravel         34.           5 782 451         Sand and gravel         34.           5 782 451         Sand and gravel         18.           5 782 651         Sand and gravel         18.           5 786 069         Sand and gravel         16.           5 786 291         Sand and gravel         45.           5 786 291         Sand and gravel         45.           5 787 652         Sand and gravel         45.           5 787 652         Sand         45.		n/a Pros	Prospective 1.0	10,000	Low topography beach ridge.
5 787 825     Till     115       5 782 823     Sand and     34,       5 782 851     Sand and     18.       5 782 451     Sand and     18.       5 782 451     Sand and     18.       5 782 581     Sand and     18.       5 782 6059     Sand and     16.       5 786 059     Sand and     16.       5 786 291     Sand and     16.       5 786 291     Sand and     16.       5 787 652     Sand and     16.       5 787 652     Sand     11.	0.1	n/a Pro	Probable 1.0	10,000	Low topography beach ridge.
5 792 833         Sand and gravel           5 782 451         Sand and gravel           5 781 577         Sand and gravel           5 786 069         Sand and gravel           5 786 291         Sand and gravel           5 786 291         Sand and gravel           5 787 253         Till           5 787 652         Sand	2.2	2.2 Pros	Prospective 2.0	> 1,000,000	> 1,000,000 Undulating till deposit. Partially mined
5 782 451         Sand and gravel           5 781 577         Sand and gravel           5 786 069         Sand and gravel           5 786 291         Sand and gravel           5 787 123         Till           5 787 652         Sand	1.3	n/a Pro	Probable 2.0	75,000	Beach ridges overlying an northeast/southwest oriented till ridge.
5 781 577         Sand and gravel           5 786 069         Sand and gravel           5 786 291         Sand           5 787 123         Till           5 787 652         Sand	0.5	0.5 Pro	Probable 1.0	50,000	Beach ridges overlying an northeast/southwest oriented till ridge. Partially mined on a small area.
5 786 069         Sand and gravel           5 786 291         Sand           5 787 123         Till           5 787 652         Sand	1.3	0.5 Pro	Probable 1.0	40,000	Beach ridges overlying an northeast/southwest oriented till ridge.
5 786 291 Sand 5 787 123 Till 5 787 652 Sand	1.1	0.4 Pro	Probable 1.0	60,000	Beach ridges overlying shallow bedrock and till.
5 787 123 Till 5 787 652 Sand	2.5	0.4 Pro	Probable 3	> 500,000	Beach ridges north/south oriented. Signicant access to build.
5 787 652 Sand	adjacent	n/a Pros	Prospective 2.5	> 250,000	Northeast/southwest oriented till ridge.
	1.7	1.4 Pro	Probable 0	0	Littoral deposit containing an important proportion of fine particles. To avoid.
357 783 5 789 444 Sand and 6.8 gravel	1.3	1.0 Pro	Probable 1.5	20,000	Beach ridges overlying an northeast/southwest oriented till ridge. Easily accessible using existing access road.
358 797 5 788 889 Sand 25	adjacent	n/a Pro	Probable 3	> 200,000	Exisiting borrow pit with signicant remaining volume. Easily accessible.
358 532 5 790 058 Till 17.4	0.2	0.2 Pros	Prospective 2.5	> 150,000	Northeast/southwest oriented till ridge.

## APPENDIX D Test Pit Reports



0	Sta	antec															Т	EST PI	T REF	POR
Proje		James	s Bay Lithium Pegn tial Borrow Source		L	ocatio	on :		TM, z 57214		8					Boreh Page :			PB	<b>S-02-</b> 1 of 1
roie	ct No	D.: <b>12162</b>	2255.220.700		Y	:		57	7665	3.2						Start d			201	1 of 1 9-08-02
lien			y Lithium (Canada)	Inc.		•••	f borehol					I				Inspec				erpaels
ite:			- , ,	81 - James Bay Road			nent :		and s	hove						Depth			WI. VC	1.60 n
igur	e:		·, ····	<b>,</b>		asing orer :			mm mm							Elevat				n
		SAMPL	Е ТҮРЕ	QUALITATIVE TERM	INOLOGY		QUA	NTITA	FIVE T	RMI	NOLO	<u>GY</u>		SYMBOLS	<u>5</u>			GROUNDW	ATER	
ss cs		Split sp		Clay	< 0.002 m 02 - 0.08 m		Traces Some					: 10 % - 20 %		dard peneti M D 1586)	ration val	ue		Date		Depth
DC			uous sampling nd rock core	Silt 0.0 Sand	0.08 - 5 m	m	Adjective				20 -	- 35 %	Nc Dyna	amic cone p		n value	Reading			m
AS TM	,	Auger Thin wa	all sampler	Gravel Cobbles	5 - 80 m 80 - 200 m		and (ex: a Main wo			omin		35 % action		2501-145) Quality De		(%)	Reading	2		m
ST M/		Shelby	tube	Boulders	> 200 m				_					·		()	Remark	s :		
IVI/	•		l sample E <u>STATE</u>	месн	ANIC CHAR			50115					ROCI	K QUALITY I	DESIGNAT			JOINTS S	DACING	
$\square$	$\triangleleft$	Remou		COMPACTION	INDEX "N		CONSIST			Cu	OR Su	(kPa)	QUALIFICA		DESIGNA	RQD	Very tig		Acinto	< 20 m
		Intact (i	thin wall sampler)	Very loose Loose	0- 4-1		Very soft Soft				1	< 12 2 - 25	Very poor Poor			< 25 % - 50 %	Tight Close			0 - 60 m - 200 m
		Lost		Compact	10 - 3	30	Firm				2	5 - 50	Fair		50	- 75 %	Modera	ately spaced	200	- 600 m
			iamond rock core)	Dense Very dense	30 - 5 > 5	50	Stiff Very stiff				100	- 100 - 200	Good Excellent			- 90 % 100 %	Spaced Very sp		2000 -	2000 m 6000 m
						1	Hard			וחו		> 200					Wide		>	6000 m
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Remoul Intact (i Lost	ded .hin wall sampler)	MECH, COMPACTION Very loose Loose Compact Dense Very dense	INDEX "I 0 - 4 - 1 10 - 1 30 - 1	N" - 4 10 30 50	CONSISTI Very soft Soft Firm Stiff	ENCY		Cu	12 25 50 100	< 12 2 - 25 5 - 50 - 100 - 200	QUALIFICA	TIVE	25 50 75	RQD < 25 % i - 50 % i - 75 % i - 90 %	Tight Close Modera Spaced	ht tely spa		6 20 600 2000	i 20 - 60 m 0 - 200 m 0 - 600 m - 2000 m - 6000 m > 6000 m
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ELEVATION (m) / DEPTH (m)			SYMBOL	STATE	TYPE N°	SUB - SAMPLE	CALIBER	RECOVERY (%)	N - RQD	pen	etration test	<ul><li>WATER LEVEL</li><li>WATER INFLOW</li></ul>	S : ser C : co W : wa W <sub>L</sub> : liq Dr : sp k : pe f'c : co OM: or	dimenton nsolidatio ater conte uid limit astic limit ecific grav rmeabilit mpressive ganic mat	netry on ent vity y e str. :ter	∀ : Nc : Cu : Cu : Cu : Su : Su W <sub>F</sub>	(dyn. intac remo intac remo W	pen.) t ulded ulded WL	REMARKS
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	Split sp. Continu Diamom Auger Thin wa Shelby i Manual SAMPLI Remoul Intact (t Lost Core (di U) HLd30 0.00 -0.05 0.05 0.55 0.55 -1.20 1.70 -1.70	Thin wall sampler Shelby tube Manual sample SAMPLE STATE Remoulded Intact (thin wall sampler) Lost Core (diamond rock core) STRATIG DESCRI DESCRI DESCRI DESCRI A DESCRI A DESCRI A DESCRI A DESCRI A DESCRI A DESCRI A DESCRI A DESCRI A DESCRI A DESCRI A A DESCRI A DESCRI A A A A A A A A A A A A A	Split spoon Continuous sampling Diamond rock core Auger Thin wall sampler SAMPLE STATE Remoulded Intact (thin wall sampler) Lost Core (diamond rock core)       Match COMPACTION Very loose Loose Compact Dense Very dense         STRATIGRAPHY         Itact (thin wall sampler) Lost Core (diamond rock core)       DESCRIPTION OF SOILS AND ROCK         SAMPLE STATE Remoulded Intact (thin wall sampler) Lost Core (diamond rock core)       DESCRIPTION OF SOILS AND ROCK         Itact (thin wall sampler) Lost Core (diamond rock core)       DESCRIPTION OF SOILS AND ROCK         Itact (thin wall sampler) Lost Core (diamond rock core)       DESCRIPTION OF SOILS AND ROCK         Itact (thin wall sampler) Lost Core (diamond rock core)       DESCRIPTION OF SOILS AND ROCK         Itact (thin wall sampler) Lost Core (diamond rock core)       Topsoil. Topsoil. - Presence of cobbles         0.00 0.005 0.005       Topsoil. Reddish-brown to brown, gravelly SAND with traces of silt. - presence of cobbles - moist         -1.20 1.20       Light brown, SAND with some gravel and traces of silt. - moist         -1.20       Light brown, SAND with traces of gravel and silt. - wet End of test pit.	Split spoon Continuous sampling Diamod rock core Auger       Clay       <0.002 n (0.002 n (0.002 - 0.08 - 5 n (0.002	Split spoon Continuous sampling Diamond rock core Auger Shelby tube Manual sampler       Clay Sand Cobles 80.002 - 0.08 mm Gravel Cobles 80 - 200 mm Boulders       0.002 - 0.08 mm 0.008 - 5 mm Gravel S - 80 mm Cobles 80 - 200 mm Boulders         SAMPLE STATE Remoulded Intact (thin wall sampler) Lost Core (diamond rock core)       INDEX "N" Very lose 0.000 Core (diamond rock core)       INDEX "N" Very lose 0.000 Dense Very dense       INDEX "N" Very dense       INDEX "N" Ve	Split spoon Continuous sampling Diamond rock core Auger Thin wall sampler SAMPLE STATE Remoulded Intact (thin wall sampler) Lost Core (diamond rock core) STRATIGRAPHY Core (diamond rock core) STRATIGRAPHY Core (diamond rock core) STRATIGRAPHY	Split spoon Continuous sampling Diamond rock core Auger Thin wall sampler Belue tube Remoulded Compact 10 - 30 Compact 10 - 30	Split spoon Continuous sampling Marvel asampler Sand 0.082-50 mm Manual sampler Sand 0.082-50 mm Manual sampler SAMPLE STATE Remoulded Cobbles 80-200 mm Manual sampler SAMPLE STATE Remoulded Core (diamond rock core) COMPACTION Very loss Core (diamond rock core) COMPACTION OF SOILS Core (diamond rock core) DESCRIPTION OF SOILS MD ESCRIPTION OF SOILS Compact 10-30 Very dense 30-50 Very dense 30-50 Very dense 30-50 Very dense 30-50 Very soft Compact 10-30 Very dense 30-50 Very dense 30-50 Very dense 30-50 Very soft Compact 10-30 Very dense 30-50 Very dense 30-50 V	Split spoon Continuous sampling Damond rock core Ager Thin wall sampler Shelby tube Manual sampler Same Cobbles Boulders Manual sampler SAMPLE STATE Remoulded Intact (thin wall sampler) Core (diamond rock core) COMPACTION Core (diamond rock core) COMPACTION COMPACTION Core (diamond rock core) COMPACTION Core (diamond rock core) COMPACTION Core (diamond rock core) COMPACTION COMPACTI	Selit spoon Continuous sampler Sand Ager Trin wall sampler Shelby tube Builders Anno Cobbles Same Cobbles Boulders Anno Cot Main word Manu sampler Same Cobbles Boulders Same Cobbles Boulders Same Cobbles Boulders Compact Lose Compact Lose Compact Lose Compact Dense STRATIGRAPHY SAND with traces of silt - presence of cobbles - moist - 200 Cobbles SAND with traces of gravel and traces of silt - moist - wet End of test pit. Same Cobbles Same Same Same Same Same Same Same Same	Solt spont       Clay       < 0.002	Solit spon Diamond rack core Name Sand Diamond rack core Name Sand	Split spon Demondratic core Demondratic core Shelby tube Shelby tu	Split spont Dimension Core Sand Subject of Core Sand Subject via Set of table Sand Sand Sand Sand Sand Sand Sand Sand	Salti spon Dimoni Actorie Salti spon Salti spon Sa	Split soon method was a split wasplit was a split was a split was a split wasp	Specification and approximate and the second specification of the second specification	Soft som mig Soft som mig Age Age Age Age Age Age Age Ag	Safe is som men Safe i

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		SAMPI	LE TYPE	QUALITATIVE TERMIN	IOLOGY		QUA		TIVE T	ERMI	NOLOO	GY		SYMBOL	S			GROU	NDW	ATER	
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AS		Auger		Gravel	5 - 80 m	m	and (ex:	and gra			>	35 %	(BNC	2501-145)					PBS 2019 M. Ve  DUNDWATER Date Dr Date		
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			(thin wall sampler)	Loose Compact	4 - 1 10 - 3	30	Soft Firm					2 - 25 5 - 50	Poor Fair			- 50 % - 75 %	Close Modera	itely spa	ced		0 - 200 m 0 - 600 m
		Lost	diama and usely as us )	Dense Very dense	30 - 5 > 5		Stiff Very stiff	F				- 100 - 200	Good Excellent			- 90 % 100 %	Spaced Very sp				- 2000 m - 6000 m
		Core (c	liamond rock core)				Hard				;	> 200					Wide				> 6000 m
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<u>ل</u>	DEPTH (ft)	ELEVATION (m) / DEPTH (m)		AND ROCK	SYN	ST	۲ ۲	n m	CALIBER	RECOVERY (%)	7		/S/150mm	VAT	k :pe	cific grav	y	W <sub>P</sub>	w	ΨL	NEX N
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lient:			/ Lithium (Canada)		E	Equipment : Hand shovel														N	1. Verp	
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lient:		Galax	y Lithium (Canada)	Inc.			f borehol nent :		lanua and s			3				Inspec	tor :		M. V	/erpaels
te:		Vicini	ty of roadstop KM3	81 - James Bay Road	Casings : mm											Depth				2.00
gure:					С	orer :			mm							Elevat	ion :			
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Project:		s Bay Lithium Pegn ntial Borrow Source		X		on :	3	TM, z 59808	3.3	18					Boreh Page :			PE	3 <b>S-09-2</b> 1 of 1		
		22255.220.700		Y		of borehol		78409 anua		npline	a				Start date : 2019-07-3						
Client: Site:		ky Lithium (Canada) ity of roadston KM3	Inc. 81 - James Bay Road	E	quipr	ment :	н	and s			5				Inspec Depth	Inspector : M. Verpaels					
igure:	Vicini	ity of roadstop rails	or - James Day Road		asing orer	-		mm mm								Depth : 1.10 Elevation :					
	SAMP	LE TYPE	QUALITATIVE TERMIN	OLOGY		QUA	NTITA	TIVE T	ERMI	NOLO	GY		SYMBOL	<u>s</u>		GROUNDWATER					
ss cs	Split s Contin	poon uous sampling		: 0.002 m	002 mm         Traces         < 10 %         N         Standard penetration valu           0.08 mm         Some         10 - 20 %         (ASTM D 1586)									ue		Date	2	Depth			
DC AS		nd rock core		0.08 - 5 m 5 - 80 m	m	Adjective	ljective (y) 20 - 35 % Id (ex: and gravel) > 35 %				- 35 %	Nc Dyna (BNC	n value	Reading Reading	g1 m						
TW ST		all sampler		0 - 200 m > 200 m	m	Main wo			Domin		action		Quality De		(%)	Remark	•	I			
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$\sim$	<u>SAMPI</u> Remou	LE STATE	MECHA COMPACTION	NIC CHAR		RISTICS O CONSIST		i	<b>c</b>	0.0 5.	ı (kPa)	ROC QUALIFIC		DESIGNA	rion RQD	Very tig		SPACING	< 20 mr		
		(thin wall sampler)	Very loose	0 -	4	Very soft			cu		< 12	Very poor			< 25 %	Tight	siic		20 - 60 mr		
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		diamond rock core)	Dense Very dense	30 - 5 > 5		Stiff Very stif	f				) - 100 ) - 200	Good Excellent			- 90 % 100 %	Spaced Very sp		2000	- 2000 mr - 6000 mr		
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	/ (u						Ľ		(%)				WATER LEVEL / WATER INFLOW	C : coi	dimenton nsolidatio	on .	▽ : Nc (dyr ■ : Cu inta	ct			
트 문	n) (m) H	DESCR	IPTION OF SOILS	SOL S	μ	å	AMP	ER	RY	- RQD		andard etration	R R R L	W. : lia	vater content iquid limit		□ : Cu remoulded ◆ : Su intact ◇ : Su remoulded		RK		
DEPTH (m) DEPTH (ft)	LEVATION (m) / DEPTH (m)		AND ROCK	SYMBOL	STATE	TYPE N°	ŝ	CALIBER	N N	- R		test	ATE ATE	Dr : spe	stic limit	/ity	ି:Surem Wୃୁୁଷ		REMARKS		
	D ELE			N N			SUB - SAMPLE	0	RECOVERY (%)	-	BLOW	VS/150mm		f'c : coi	rmeabilit npressive ganic mat	e str.	F-C	•	R		
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Proje	ect No	D.: <b>12162</b>	2255.220.700		Y	:	fhorobol	57	78389	3.6		_				Start d	ate :		201	9-07-31		
			,				nent :		anual sampling and shovel							Inspect		M. Verpaelst				
		Vicini	31 - James Bay Road		asing			mm							Depth Elevati			1.05 m m				
- igu	0.	: James Bay Lithium Pegmatite Project - Potential Borrow Source Assessment         No.: 121622255.220.700 Galaxy Lithium (Canada) Inc. Vicinity of roadstop KM381 - James Bay Road         SAMPLE TYPE Split spoon Continuous sampling Diamond rock core Auger Thin wall sampler Shelby tube Manual sample       QUALITATIVE TERM Clay Sand Gravel Cobles Boulders         SAMPLE STATE Remoulded Intact (thin wall sampler) Lost Core (diamond rock core)       MECH COMPACTION Very loose Lose Compact Dense         Core (diamond rock core)       DESCRIPTION OF SOILS AND ROCK         DESCRIPTION OF SOILS AND ROCK       ONO Dense         0.00       Brown, SAND and gravel with traces of silt and clay. - presence of cobbles         -0.60       Brown, SAND and gravel with traces of silt.         -0.60       Brown, SAND and gravel with traces of silt.         -1.05       Auger refusal on probable gravel or cobble.				orer			mm	CDAAI		CV.		SYMBOL		2.0144		GROUNDW	ATED			
		Split sp	boon	Clay < 0.0	002 mi		Traces	IANTITATIVE TERMINOLOGY < 10 %				< 10 %		dard penet	-	ue		GROUNDWATER				
D	:	Diamo		Sand 0.08	- 5 mi	n	Some Adjective					- 35 %	% (ASTM D 1586) % Nc Dynamic cone pene			on value	Reading	g 1 Date Depth				
<u>л</u>	v	Thin w		Cobbles 80 - 2	80 mi 200 mi	n	and (ex: a Main wo			omin		> 35 % action		2501-145) Quality De		(%)	Reading			m		
				Boulders > 2	200 mi	n											Remark	5:				
				MECHANIC			RISTICS OF	FSOILS	<u>i</u>				ROCI	K QUALITY	DESIGNA	TION		JOINTS SI	PACING			
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			thin wall sampler)		4 - 1 10 - 3		Soft Firm					2 - 25 5 - 50	Poor Fair			- 50 % - 75 %	Close Modera	ately spaced		- 200 mm - 600 mm		
			liamond rack cara)		30 - 5 > 5		Stiff Very stiff					) - 100 ) - 200	Good Excellent			- 90 % - 100 %	Spaced Very sp			2000 mm 6000 mm		
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			STRATIG	KAPHY				S		ΠPI	LES	5		~ >	GA · arr	ain size an		ESTS X:N (standa	rd nen V			
		1						щ		(%				WATER LEVEL / WATER INFLOW	S : see	dimentom nsolidatio	etry		pen.)			
Ξ	Potential Borrow Source Assessment         Project No.:       121622255.220.700         Client:       Galaxy Lithium (Canada) Inc.         Site:       Vicinity of roadstop KM381 - James Bay Road         Figure:       QUALITATIVE         SS       Split spoon         CS       Continuous sampling       Clay         DC       Diamond rock core       Sand         Age       Auger       Gravel         TW       Thin wall sampler       Boulders         MA       Manual sample       Combolies         SAMPLE STATE       COMPACTION       Very loose         Lost       Description of core (diamond rock core)       Description of soils         Desc       Description of soils       Desc         Out       Brown, SAND and gravel with tra       On of silt and clay.       - presence of cobbles         Out       Brown, SAND and gravel with tra       Of silt.       - 1.05         Out       Brown, SAND and gravel with tra       Of silt.		Ы	ш	å	SUB - SAMPLE	К	RECOVERY (%)	a		andard etration	IN LE	W :wa	iter contei uid limit		☐ : Cu remot ♦ : Su intact	remoulded					
HH	РТН	ATIO PTH			SYMBOL	STATE	TYPE N°	- SA	CALIBER	N	- RQD		test	TER	W <sub>p</sub> :pla	astic limit ecific grav	ity	$\diamond$ : Su remou	lded	REMARKS		
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lient:			y Lithium (Canada)	Inc.		•••	of borehol										Inspector : M. Verpael					
ite:	١	Vicinit	y of roadstop KM3	81 - James Bay Road		asing	ment : gs :		and s mm	snove	21						Depth : 1.20					
igure:					C	Corer	:		mm							Elevat	tion :				m	
SS		AMPLE plit spo		QUALITATIVE TERM	<u>1INOLOGY</u> < 0.002 m		<u>QUA</u> Traces	NTITA	TIVE T	ERMI	NOLOG	<u>Y</u> 10 %	N Stan	<u>SYMBOL</u> dard penet		lua	GROUNDWATER					
cs	c	ontinu	ous sampling		002 - 0.08 m	ım	Some				10 - 2	20 %	(AST	M D 1586)			Date Depth					
DC AS	Α	uger	d rock core	Sand Gravel	0.08 - 5 m 5 - 80 m	ım		and gra	nd gravel) > 35 % (BNQ 2501-145)						)			Reading 1 m Reading 2 m				
TW ST	S	helby t		Cobbles Boulders	80 - 200 m > 200 m		Main wo	ord	Ľ	omin	ant frac	ction	RQD Rock	Quality De	signation	1 (%)	Remark	:				
MA			sample <u>E STATE</u>	MECI	HANIC CHAF				:				ROC					IOINT	S SPACI	NG		
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	l Ir	ntact (t	hin wall sampler)	Very loose Loose	0 - 4 - 1	- 4 10	Very soft Soft	t			12	< 12 - 25	Very poor Poor		25	< 25 % 5 - 50 %	Tight Close			20 - 60 60 - 200		
	L	ost		Compact Dense	10 - 30 -		Firm Stiff					- 50 - 100	Fair Good			) - 75 % i - 90 %	Modera Spaced	itely space		200 - 600 00 - 2000		
	] c	ore (di	amond rock core)	Very dense	>	50	Very stif Hard	f			- 100 - >	200	Excellent		90 -	- 100 %	Very sp Wide	aced	20	00 - 600 600 <		
			STRATIG	RAPHY				S	AN	ΙP	LES	;					T	ESTS				
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프 프		2 2 7		IPTION OF SOILS	SYMBOL	STATE	TYPE N°	MAS	BE	ER	- RQD	pene	etration test	ER L	W. : lia	uid limit astic limit		♦ : Su int ♦ : Su rer	act		ARK	
DEPTH (m) DEPTH (ft)	DESCRIPTION OF SOILS AND ROCK		SYN	ST	ΤΥΡ	SUB - SAMPLE	CALIBER	RECOVERY (%)			/S/150mm	WATER LEVEL / WATER INFLOW	Dr:sp k:pe	ecific grav rmeabilit	vity ty	W <sub>P</sub> V	w w		REMARKS			
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ss cs		Split sp	oon Ious sampling		0.002 m		Traces Some					: 10 % - 20 %		dard penet M D 1586)	ration val	ue		Date		Depth
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ST MA		Shelby Manua	tube sample	Boulders	> 200 m	m											Remarks	5:		
			E STATE	MECHAN	C CHAR	ACTE	RISTICS O	F SOILS					ROC	K QUALITY	DESIGNA	<u>FION</u>		JOINTS	SPACING	<u>i</u>
$\geq$	]	Remou	ded		NDEX "N		CONSIST			Cu	OR Su	(kPa)	QUALIFIC			RQD	Very tig	ht		< 20 m
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		Lost		Compact Dense	10 - 3 30 - 5		Firm Stiff					5 - 50 - 100	Fair Good			- 75 % - 90 %	Modera Spaced	itely spaced		0 - 600 m - 2000 m
		Core (d	iamond rock core)	Very dense	> 5		Very stiff Hard				100	- 200 > 200	Excellent			100 %	Very sp Wide		2000	- 6000 m > 6000 m
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<u> </u>		EE		IPTION OF SOILS AND ROCK	SYMBOL	STATE	TYPE N°	SA	CALIBER	Ň	- RQD		test	ШШ	W, pla	stic limit cific grav	itv	$\diamond$ : Su rem	oulded	REMARKS
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	SAMP	LE TYPE	QUALITATIVE TERMINO	DLOGY		QUA	NTITA	TIVE T	ERMI	NOLOG	<u> 34</u>		SYMBOL	<u>s</u>	I		GROUN	DWATER	
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	Lost		Dense Very dense	30 - 5 > 5	50	Stiff Very stiff				50	- 100 - 200	Good Excellent		75	5 - 90 % - 100 %	Spaced Very sp		600	- 2000 m - 6000 m
	Core (	diamond rock core)			,	Hard				;	> 200	Execution			100 /0	Wide			> 6000 m
	1	STRATIO	GRAPHY				S	AN	IPI	LES	5		2	GA · gr:	ain size a		ESTS	ndard pen.	)
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Mail angle     Index 1 (Sold Participant)     Index	W T						Main wo	rd	Ď	omin	ant fr	action	RQD Rock	Quality De	signation	(%)					
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Instruct (bin wall samplar) tort         Very loss borne Dome         0 - 4 13 b - 58 borne Dome         Very loss 3 - 58 borne 3 - 58 break         0 - 4 13 b - 58 break         Very loss break         Ve									<u>.</u>	<b>C</b> 11	0.0 5.	(48-2)			DESIGNAT		Voryti		INTS	SPACING	
tox:       Compare:       10 - 30 - 10 - 10 - 10 - 10 - 10 - 10 -				Very loose	0 -	4	Very soft			cu		< 12	Very poor			< 25 %	Tight	Bur			20 - 60 n
Tore (damond rock and live)       Way stiff       30:20       Deallert       90:100       Way stiff       200:200       Way stiff       30:20       Way stiff       Way			unin wan sampler)	Compact	10 - 3	30	Firm				2	5 - 50	Fair		50	- 75 %	Moder		aced	20	0 - 600 n
STRATIGRAPHY     SAMPLES     TESTS       i ging i			liamond rock core)								100	- 200					Very sp			2000	- 6000 n
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0000       Reddishbown, GRAVEL and sand with traces of situal diags, - presence of cobbies         0055       End of test pit.		0.00			/	ŕ									CA : che	emical an	alyses				
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	Gala	xy Lithium (Canada)	Inc.			f borehol					)				Inspec	tor :			Verpaelst
igure:	Vicin	ity of roadstop KM3	81 - James Bay Road		quipri asing	nent: s:		and s mm	hove	1					Depth	:			0.65 n
					orer :			mm							Elevati	ion :			n
	SAMP	LE TYPE	QUALITATIVE TERMINO	LOGY		QUA	NTITA	TIVE T	ERMI	NOLO	GY		SYMBOL	S			GROUND	WATER	
SS	Split s	poon	Clay <0	.002 m		Traces				<	< 10 %		dard penet		ue				
CS DC		nuous sampling and rock core	Silt 0.002 - Sand 0.0	0.08 m 8 - 5 m		Some Adjective	e (v)				- 20 % - 35 %		M D 1586) mic cone p	enetratio	n value	Reading	Date	;	Depth m
AS	Auger		Gravel 5	- 80 m	m	and (ex:	and gra			>	<b>&gt; 35 %</b>	(BNC	2501-145)			Reading			m
TW ST	Shelby	/all sampler / tube		200 m 200 m		Main wo	ora	L	omin	ant fra	action	KQD KOCK	Quality De	signation	(%)	Remark	5:		
MA	Manua	al sample																	
		<u>LE STATE</u>	MECHANI											DESIGNAT				SPACING	
$\geq$	Remo		COMPACTION IN Very loose	N" IDEX اا - 0		CONSIST Very soft			Cu	OR Su	(kPa) < 12	QUALIFICA Very poor			RQD < 25 %	Very tig Tight	sht		< 20 mr 20 - 60 mr
	Intact	(thin wall sampler)	Loose	4 - 1	.0	Soft					2 - 25	Poor		25	- 50 %	Close		6	0 - 200 mr
	Lost		Compact Dense	10 - 3 30 - 5	0	Firm Stiff					25 - 50 ) - 100	Fair Good			- 75 % - 90 %	Spaced		600	0 - 600 mr - 2000 mr
	Core (	diamond rock core)	Very dense	> 5		Very stiff Hard	f				) - 200 > 200	Excellent		90 -	100 %	Very sp Wide	aced		- 6000 mr > 6000 mr
		STRATIG	RAPHY			Taru	S	ΔΛ	١PI	LES							ESTS		2 0000 1111
													Z≥		in size an	alysis	imes : N (stan		.)
	ì						Щ		(%)				202	C : cor	limentom nsolidatio	n Í	⊽ : Nc (dyr ∎ : Cu inta		
E E	ΣÊ			Р	щ	å	MP	R	ž	R		andard etration	E E	W :wa W :liqu	ter conte Jid limit	nt	□: Cu rem ♦: Su inta		N N
듣  푼	EF		IPTION OF SOILS	SYMBOL	STATE	TYPE N°	SA	CALIBER	۲ 	- RQD		test	世世	W ;; pla	stic limit cific grav	,itu	🛇 : Su rem	oulded	¥
DEPTH (m) DEPTH (ft)	EVATION (m) / DEPTH (m)			sγ	ò	∣≿	SUB - SAMPLE	ຽ ບ	RECOVERY (%)	2	BLOW	/S/150mm	WATER LEVEL / WATER INFLOW	k :pe	meability	v	w,w ⊢⊂	W.	REMARKS
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	0.00	Topsoil.		~~~	:									CA : che	emical an	alyses		+ + + +	
	-0.10		, SAND with traces of	100	e T														_
	0.10	├ silt.		/		-													
	0.25	∖ - moist	/		IV	MA-1													_
		Reddish-browi traces of silt.	n, gravelly SAND with		A														
-	-0.65 0.65		cobbles and boulders	<u>م الم</u>	<u> </u>														
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The start (Minimum (Leining))         Construction         Engingere: mm	roject	No.	.: 12162	22255.220.700				- <b>f</b> h h - l									Start d	late :				2019	9-08-0
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Signal space         Care 1000000000000000000000000000000000000			SAMPL	LE TYPE	QUALITATIVE TERMI	NOLOGY		QUA		TIVE T	ERMI	INOLO	GY		SYMBOL	S			GROL	JNDV	VATE	R	
Characteristics         State         Construction         State	SS		Split sp	poon	Clay	< 0.002 r		Traces					< 10 %		dard penet		ue						
Solution     assumption     image: im	CS DC								e (v)							enetratio	n value	Reading		Date	-		
Bit         Bodder:         > 20 mm         Fordation         Fordation           Minute (bit) with simple in the (bit) with simple in t	AS		Auger		Gravel	5 - 80 r	nm	and (ex:	and gra			;	> 35 %	(BNC	Q 2501-145)								
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Total (thin wall sample)         Local wall sample)         Local wall wall wall wall wall wall wall w	$\geq$	3	Remou	lded							Cu	OR SL							ght				< 20 m
Lett       Denix       30 - 30       S0 - 30       Denix       79 - 100 %       Specied       000 - 000       So - 200       So - 200<		2	Intact	(thin wall sampler)	Loose	4 -	10	Soft					L2 - 25	Poor		25	- 50 %	Close				60 -	200 m
Tore diamond rack row       Very dense       > 200       President       100       200       President       2000       2000       200			Lost																	ced			
STRATIGRAPHY SAMPLES TESTS TOTAL TOT		٦	Core (c	diamond rock core)				Very stif	f			100	) - 200					Very sp				00 - 6	6000 m
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understand       understand <td></td> <td></td> <td></td> <td>STRATIG</td> <td>FRAPHY</td> <td></td> <td></td> <td></td> <td><u> </u></td> <td>5AN</td> <td>ЛP</td> <td>ĻE</td> <td><u>s</u></td> <td></td>				STRATIG	FRAPHY				<u> </u>	5AN	ЛP	ĻE	<u>s</u>										
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6.02       Redoutshown to drown, gravely         . presence of cobbles and boulders         . indurated         . induction         . induction     <		١	0.00 /	Topsoil.		/11:	7	-								CA : che	emical an	alyses		+ -			
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1.40       Grey, gravely sand with some sit.         - presence of cobbles and boulders         - moist         End of test pit.         10         11         11         11         11         11         11         11         11         11         11         11         11         11         11         11         12         13         14	1							(   MA-1															
1.40       Grey, gravely sand with some sit.         - presence of cobbles and boulders         - moist         End of test pit.         10         11         11         11         11         11         11         11         11         11         11         11         11         11         11         11         12         13         14																							
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5       -1.50 1.50       - presence of cobbles and boulders - moist End of test pit.         10       -         11       -         10       -         10       -         10       -         11       -         11       -         11       -         11       -         11       -         11       -         11       -         11       -         11       -         11       -         11       - <tr< td=""><td></td><td></td><td>1.40</td><td>Grev. gravelly</td><td>sand with some silt.</td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr<>			1.40	Grev. gravelly	sand with some silt.			-															
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igu	e:					orer			mm							Elevati	ion :			n
		SAMPI	LE TYPE	QUALITATIVE TERMINO	.OGY		QUA	ANTITA	TIVE T	ERMI	NOLO	GY		SYMBOL	S			GROUND	WATER	
SS		Split s	poon	Clay <0	.002 m		Traces					< 10 %		dard penet		ue				<u> </u>
CS DC			uous sampling nd rock core	Silt 0.002 - Sand 0.0	0.08 m 8 - 5 m		Some Adjective	e (v)				- 20 % - 35 %		M D 1586) amic cone p	enetratio	on value	Reading	Dat 1	e	Depth m
AS		Auger		Gravel 5	- 80 m	m	and (ex:	and gra			:	> 35 %	(BNG	Q 2501-145)			Reading			m
TV ST		Shelby			200 m 200 m		Main wo	ora	L	Jomin	iant fr	action	RUD ROCH	Quality De	signation	(%)	Remark	s :		
M	4		al sample																	
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	$\leq$	Remou		COMPACTION IN Very loose	۱DEX "N - 0		CONSIST Very soft			Cu	OR SI	ı (kPa) < 12	QUALIFIC/ Very poor			RQD < 25 %	Very tig Tight	gnt		< 20 mi 20 - 60 mi
		Intact	(thin wall sampler)	Loose Compact	4 - 1 10 - 3		Soft Firm					12 - 25 25 - 50	Poor Fair			- 50 % - 75 %	Close Moder:	ately spaced		50 - 200 mi 10 - 600 mi
		Lost		Dense	30 - 5	50	Stiff	_			50	) - 100	Good		75	- 90 %	Spaced		60	) - 2000 mi
		Core (d	diamond rock core)	Very dense	>5	50	Very stif Hard	f			100	) - 200 > 200	Excellent		90 -	- 100 %	Very sp Wide	baced	200	) - 6000 mi > 6000 mi
			STRATIG	RAPHY				S	<b>SAN</b>	ΛP	LE	S					Т	ESTS		
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اح	-	ELEVATION (m) / DEPTH (m)						SUB - SAMPLE		RECOVERY (%)		-		WATER LEVEL / WATER INFLOW	C : coi	nsolidatio	on .	🔳 : Cu inta	act	
DEPTH (m)	E T	ΞĒ	DEADD		٦ ۵	Ľ	, °z	μ,	ER	R	B		andard etration		W. : lia	iter conte uid limit		□ : Cu ren ♦ : Su inta		ž
È	DEPTH (ft)	PTA		IPTION OF SOILS	SYMBOL	STATE	TYPE N°	-s	CALIBER	<u>ا</u>	- RQD		test	巴臣	W <sub>p</sub> :pla	astic limit ecific grav	/itv	🛇 : Su ren		REMARKS
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		<b>田</b> 0.00						S		2				¥ 🔶	OM: org	ganic mat	ter	20 40	60 80	
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		\0.05 0.05		n, gravelly SAND with																
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Sta	antec															-	TEST PIT I	REPOR
Project:	Jame	s Bay Lithium Pegn Itial Borrow Source	natite Project - Assessment	X		on :	3	TM, z 54851	1.9	18					Boreh Page	ole :		PBS-18- 1 of
Project No	D.: <b>12162</b>	22255.220.700		Y		fhaucha		78610							Start o	late :		2019-08-0
lient:		(y Lithium (Canada)			•••	of boreho ment :		lanua land s			3				Inspec			M. Verpaels
Site:	Vicini	ity of roadstop KM3	81 - James Bay Road	С	asing	gs :		mm							Depth			0.90
igure:			1	С	orer	:		mm							Elevat	ion :		
~~		<u>E TYPE</u>	QUALITATIVE TERMINO				ANTITA	TIVE T	ERMI				SYMBOL	-			GROUNDWAT	<u>ER</u>
SS CS		uous sampling	Silt 0.002	0.002 m - 0.08 m	m	Traces Some				10	< 10 % - 20 %	(AST	dard penet M D 1586)				Date	Depth
DC AS	Diamo Auger	nd rock core		08 - 5 m 5 - 80 m		Adjectiv and (ex:		avel)			- 35 % > 35 %		amic cone p Q 2501-145)		on value	Reading Reading		m m
TW ST	Thin w Shelby	all sampler		- 200 m > 200 m		Main wo			omin	ant fr	action	RQD Rock	Quality De	signatior	(%)	Remark		
MA		al sample	boulders	200 m												Remain		
		<u>E STATE</u>				RISTICS O		5					K QUALITY I	DESIGNA			JOINTS SPAC	
	Remou		COMPACTION I Very loose	NDEX "N - 0		CONSIST Very sof			Cu	OR Sı	ı (kPa) < 12	QUALIFICA Very poor			RQD < 25 %	Very ti Tight	ght	< 20 m 20 - 60 m
	Intact	(thin wall sampler)	Loose Compact	4 - 1 10 - 3	LO	Soft Firm					l2 - 25 25 - 50	Poor Fair			- 50 % - 75 %	Close	ately spaced	60 - 200 m 200 - 600 m
	Lost		Dense	30 - 5	50	Stiff				50	) - 100 ) - 200	Good		75	- 90 %	Spaced	3	600 - 2000 m 000 - 6000 m
	Core (c	liamond rock core)	Very dense	> !	50	Very stif Hard	t				> 200	Excellent		90	- 100 %	Very s Wide	paced 2	000 - 6000 m > 6000 m
		STRATIO	RAPHY			1	S	<b>AN</b>	/IPI	ĻE	S						ESTS	•
E I	_EVATION (m) / DEPTH (m)						SUB - SAMPLE	2	RECOVERY (%)		Sta	andard	WATER LEVEL / WATER INFLOW	S :se C :co	ain size an dimenton nsolidatio Iter conte	netry on	<ul> <li>× : N (standard</li> <li>▽ : Nc (dyn. per</li> <li>■ : Cu intact</li> <li>□ : Cu remoulde</li> </ul>	.)
DEPTH (m) DEPTH (ft)	THON THON		IPTION OF SOILS	SYMBOL	STATE	TYPE N°	SAM	CALIBER	/ER)	- RQD	pen	etration test	ER I	W, : pla	uid limit Istic limit		<ul> <li>◆ : Su intact</li> <li>◇ : Su remoulde</li> </ul>	2
	DEP	,		SYI	ST	≿	8	S	S S		вгом	/S/150mm		k :pe	ecific grav rmeabilit	y İ	w, w w	
	Ξ						ร		L R				¥ 🔶	OM: or	mpressiv ganic mat	ter	20 40 60	
	0.00	Topsoil.		7	ń	-								CA : ch	emical an	alyses		
	\0.05 0.05		n, gravelly SAND with		$\left  \right\rangle$	/												
-		traces of silt.	cobbles and boulders		V	MA-1												
		- partially indu				IVIA-1												
_	-0.70			a	/ \													
	0.70	Grey-brown, S	AND with traces of silt		Y Y	1												
_	-0.90 0.90	and gravel.			ł													
1	0.50	<ul> <li>presence of e End of test pit.</li> </ul>		/														
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Project		James	s Bay Lithium Pegn tial Borrow Source		L	ocatio	on :		TM, z 53318	one ' 3.1	18					Boreh Page			PB	S-19-1 1 of 1
roiect	t No.	.: 12162	2255.220.700		Y	':		57	78669	96.7						Start o			201	19-08-04
lient:			y Lithium (Canada)	Inc.			of borehol					3				Inspec				erpaelst
ite:				81 - James Bay Road		quipr asing	ment :			shove	el					Depth				2.05 n
igure:						orer			mm mm							Elevat	tion :			n
		SAMPL		QUALITATIVE TER				NTITA	TIVE T	ERMI				SYMBOLS				GROUNDW	ATER	
SS CS		Split sp Continu	oon Jous sampling	Clay Silt 0	0.002 m > 0.002 m		Traces Some					< 10 % - 20 %	N Stan (AST	dard penetı M D 1586)	ration val	ue		Date	D	Depth
DC		Diamor	nd rock core	Sand	0.08 - 5 m	m	Adjective				20	- 35 %	Nc Dyna	amic cone p		n value	Reading			m
AS TW		Auger Thin wa	all sampler	Gravel Cobbles	5 - 80 m 80 - 200 m		and (ex: Main wo			Domin		> 35 % action		2501-145) Quality De		(%)	Reading	<u>5</u> 2		m
ST MA		Shelby Manua	tube I sample	Boulders	> 200 m	m											Remark	s :		
			E STATE	MEC	CHANIC CHAR		RISTICS O	F SOILS	i				ROC	K QUALITY I	DESIGNAT	TION		JOINTS S	PACING	
$\geq$	3	Remou	lded	COMPACTION	INDEX "I	٧"	CONSIST	ENCY		Cu	OR Su	ı (kPa)	QUALIFICA			RQD	Very ti	ght		< 20 m
<i>\\\\\</i>	7	Intact (	thin wall sampler)	Very loose Loose	0- 4-:		Very soft Soft	t			1	< 12 12 - 25	Very poor Poor			< 25 % - 50 %	Tight Close			0 - 60 m - 200 m
		Lost		Compact	10 - 3	30	Firm				2	25 - 50	Fair		50	- 75 %	Moder	ately spaced	200 -	- 600 m
			iamond rock core)	Dense Very dense	30 - ! > !		Stiff Very stif	f			100	) - 100 ) - 200	Good Excellent			- 90 % 100 %	Spaced Very sp		2000 - (	2000 m 6000 m
		core (u				-	Hard					> 200					Wide		>(	6000 m
	-1		STRATIC	GRAPHY				S	<b>AN</b>	ΛPI	LES	S		~ >	<u></u>			ESTS		1
		_						ш		()				LOV LOV	S : sec	in size an limenton	netry	▽:Nc (dyn.	pen.)	
Ê	Ê	ELEVATION (m) / DEPTH (m)			L		<u>•</u>	- SAMPLE	2	RECOVERY (%)		Sta	andard	WATER LEVEL / WATER INFLOW	W :wa	nsolidatio ter conte		: Cu intact		S
DEPTH (m)	Ĕ	N N N N N N N N N N N N N N N N N N N	DESCR	IPTION OF SOILS	BO	Ē	Z Ш	AN	B	ER	- RQD	pen	etration	I H	W <sub>L</sub> : liqu W <sub>P</sub> : pla	uid limit stic limit		♦ : Su intact ♦ : Su remout		L A
	рертн (tt)	EP1	A	AND ROCK	SYMBOL	STATE	TYPE N°		CALIBER	S	2		test	ATI	Dr:spe k:pe	ecific grav	vity	₩, W		REMARKS
ין ב	ב				0,			SUB		E E E	-	BLOW	VS/150mm		f'c :cor	npressiv	e str.			Ř
		0.00						••						¥ +-		anic mat mical an		20 40 6	0 80	
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1	5		and traces of s	silt.	el Plus															
			- presence of o	cobbles		ł														
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		tial Borrow Source	Assessment	X				52925 78633							Page :				1 of 1
		2255.220.700	- In a			f borehol				pling	1				Start d				)19-08-04
ient: te:		y Lithium (Canada)	) Inc. 81 - James Bay Road			nent :		and s	hove	el -					Inspec Depth			M. V	/erpaels 1.50
gure:	VICIII	ty of roadstop Kint	or - James Day Road		asing orer			mm mm							Elevati				1.50
	SAMPL	Е ТҮРЕ	QUALITATIVE TERMINO				NTITA		ERMI	NOLO	GY		SYMBOL	s			GROUNI	OWATER	
SS	Split sp	oon	Clay <	0.002 m		Traces					× 10 %		dard penet		ue				Dawth
CS DC		uous sampling nd rock core	Sand 0.	- 0.08 m .08 - 5 m	m	Some Adjective				20 ·	- 20 % - 35 %	Nc Dyna	M D 1586) amic cone p		n value	Reading	Dat g 1	e	Depth m
AS TW	Auger Thin w	all sampler		5 - 80 mi - 200 mi		and (ex: a Main wo			omin		> 35 % action		2501-145) Quality De		(%)	Reading	g 2		m
ST MA	Shelby			> 200 m									<i>i</i>		,	Remark	:s :		
		E STATE	MECHAN	IIC CHAR	ACTE	RISTICS O	F SOILS	<u>i</u>				ROCI		DESIGNA	<u>TION</u>		JOINT	S SPACING	1
$\ge$	Remou	lded	COMPACTION Very loose	INDEX "N - 0		CONSIST Very soft			Cu	OR Su	(kPa)	QUALIFICA	TIVE		RQD	Very ti Tight	ght		< 20 m 20 - 60 m
	Intact (	thin wall sampler)	Loose	4 - 1	0	Soft					< 12 2 - 25	Very poor Poor		25	< 25 % - 50 %	Close		6	0 - 200 m
	Lost		Compact Dense	10 - 3 30 - 5		Firm Stiff					5 - 50 - 100	Fair Good			- 75 % - 90 %	Moder Spaced	ately spaced I		0 - 600 m - 2000 m
	Core (c	liamond rock core)	Very dense	> 5		Very stiff Hard	F			100	- 200 > 200	Excellent			100 %	Very s Wide		2000	- 6000 m > 6000 m
		STRATIO	RAPHY				S	AN	۱PI	LES							ESTS		
		•••••					1						SĽ	GA:gra S:seo	in size an limentor	alysis		ndard pen.	.)
	) (m						Ľ	~	(%)		644	andard		C : coi	nsolidatio ter conte	n Í	🔳 : Cu inta	act	S
H (the second se	NO NO	DESCR	IPTION OF SOILS	30L	Щ	ž	AM	B	۲.	- RQD	pen	etration	RL	W, : liq	uid limit		□ : Cu ren ♦ : Su inta	act	X
DEPTH (ft)	(AT)		AND ROCK	SYMBOL	STATE	TYPE N°	ŝ	CALIBER	N N	N - R		test	WATER LEVEL / WATER INFLOW	Dr : spe	stic limit cific grav	ity	⇔:Suren W <sub>P</sub> V		REMARKS
	ELEVATION (m) / DEPTH (m)			S			SUB - SAMPLE	0	RECOVERY (%)	-	BLOW	/S/150mm		f'c : coi	meability npressive	str.	⊢ ⊢∈	)—I-	
	0.00	<b>T</b>		~~~									¥ -		anic mat mical an		20 40	60 80	
	0.00	Topsoil. Grev SAND v	vith traces of silt.															_	
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			22255.220.700		Y   T		of borehol		78609 anua		npling					Start o				2019-08-04
Clier Site:			xy Lithium (Canada) uity of roadstop KM3	) Inc. 81 - James Bay Road	E	quipr	ment :	н	and s							Inspec Depth			м	Verpaels. 2.25 r
igu			,,	····		asing orer			mm mm							Elevat				r
		SAMP	LE TYPE	QUALITATIVE TERMINOL	<u>OGY</u>		-	NTITA	TIVE T	ERMI	NOLO	<u>SY</u>		SYMBOL				GROUNI	OWATER	
SS CS			nuous sampling	Silt 0.002 - 0	002 m ).08 m		Traces Some				10 -	10 % 20 %	(AST	dard penet M D 1586)				Dat	e	Depth
D( AS	5	Auger		Gravel 5	3 - 5 m - 80 m	m	Adjective and (ex:	and gra			>	35 % 35 %	(BNC	amic cone p Q 2501-145)			Reading Reading			m m
TV ST M	•	Shelb	vall sampler y tube al sample		200 m 200 m		Main wo	rd	D	omin	ant fra	action	RQD Rock	Quality De	signation	(%)	Remark	s :		
			LE STATE	MECHANIC											DESIGNA				S SPACIN	
		Remo	ulded (thin wall sampler)	Very loose	DEX "N - 0	4	CONSIST Very soft			Cu	OR Su	< 12	QUALIFICA Very poor			RQD < 25 %	Very ti Tight	ght		< 20 m 20 - 60 m
		Lost		Loose Compact	4 - 1 10 - 3	30	Soft Firm				2	2 - 25 5 - 50	Poor Fair		50	- 50 % - 75 %		ately space	1 2	60 - 200 m 00 - 600 m
			diamond rock core)	Dense Very dense	30 - 5 > 5		Stiff Very stiff	F			100	- 100 - 200	Good Excellent			- 90 % 100 %	Spaced Very sp			0 - 2000 m 0 - 6000 m
_			STRATIO	RAPHY			Hard	S	ΔΝ		LES	> 200					Wide T	ESTS		> 6000 m
												,		ŽŽ		in size a	nalysis	imes: N (sta		n.)
2	9	) m			Ι.			Ë	~	(%)		644	andard	WATER LEVEL / WATER INFLOW	C : co	dimentor nsolidation ter conto	on Ó	⊽:Nc (dy ∎:Cu int	act	S
DEPTH (m)	DEPTH (ft)	ELEVATION (m) / DEPTH (m)	DESCR	IPTION OF SOILS	SYMBOL	STATE	TYPE N°	SUB - SAMPLE	CALIBER	RECOVERY (%)	- RQD	pen	etration	1 €		uid limit		□ : Cu rer ◆ : Su inta ◇ : Su ren	act	REMARKS
Б	EPI	CAT		AND ROCK	SYM	ST/	¥	о - 0	BL	õ	7		test	ATE	Dr : sp	ecific gra rmeabilit	vity	W, V	v w	EM)
								su		RE		BLOW	/S/150mm	× ≤ ₹ ←	OM: org	npressiv ganic mat	tter	20 40	60 80	
		0.00	Topsoil.	/	1	T	-								CA : ch	emical ar	nalyses			1
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Constrained from Source Assessment     Constrained f				matite Project	1	ocot:	on :		тм -	onci	18					Boroh		EST	11		
Victure         SP2901 5	Potential Borrow Source Assessment					::	011 :				Borehole :         PBS-21-1           Page :         1 of 1										
Micro With United (Lights) RMT     Displayment     Displaymen	roject N	lo.: <b>12</b>	1622255.220.700		Y	:						-				2019					
E         Weithy of couldeby MXIII - James Big Road         Costop: Lossing: Castop: Status         Doth: Table - Status         Doth: Deckin:         3.30           5         Status         Castop: Status         Castop: Status         Status         Castop: Status         Status	lient:		•														tor :				
Monta Intri Sector         Contact Intri Sector <thcontact intri<br="">Sector         Contact In</thcontact>	ite:	Vie	cinity of roadstop KM	381 - James Bay Road																	3.30
St.         St. <td>gure:</td> <td></td> <td></td> <td></td> <td>C</td> <td>orer</td> <td>:</td> <td></td> <td>mm</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>Elevat</td> <td>ion :</td> <td></td> <td></td> <td></td> <td></td>	gure:				C	orer	:		mm							Elevat	ion :				
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St. M. Marger another break of the set	cs	Cor	ntinuous sampling	Silt 0.002	0.08 m	m	Some				10 -	20 %	(AST	M D 1586)			Decilies		te		
St.     Stephy take Monuscience     Souther     200 mm     Remotive Monuscience     Remotive Stephy take	AS	Aug	ger	Gravel	5 - 80 m	m	and (ex:	and gra			>	35 %	(BNC	2501-145	)						
Nemetic TATI Mediate         Mediate Calacity Calacity of solution must be an approximation of solution with the solution of the solution of the solution of solution of the s	TW ST						Main wo	rd	D	omin	ant fra	action	RQD Rock	Quality De	esignatior	(%)	Remark	s :			
Memoralised Interact (bit well sample) tor     COMPACTION (bit sector) Compact (bit well sample) (bit sector)     COMPACTION (bit sector)     NODEX***     CONSISTENT     Curl of bit (bit sector)     Curl of bit (bit sector) <td>MA</td> <td>Ma</td> <td>inual sample</td> <td></td>	MA	Ma	inual sample																		
Instruct (bin wait sample)         Very lease bees to be 1 = 10 bees t									<u>i</u>	<b>c</b>	<u> </u>	(kDa)			DESIGNA		Vonuti		rs spac		< 70 m
tort     Compact     20 - 30     Find     30 - 30     Find     30 - 30     Find     50 - 30     Find     Find <t< td=""><td></td><td></td><td></td><td>Very loose</td><td>0 -</td><td>4</td><td>Very soft</td><td></td><td></td><td>Cu</td><td></td><td>&lt; 12</td><td>Very poor</td><td></td><td>-</td><td>&lt; 25 %</td><td>Tight</td><td>gnit</td><td></td><td>20</td><td>- 60 m</td></t<>				Very loose	0 -	4	Very soft			Cu		< 12	Very poor		-	< 25 %	Tight	gnit		20	- 60 m
Text (diamend rock cold)       Very detti       30 200       Livelient       00 200       Very detti       30 200       Livelient       00 200       Very detti       30 200       Very detti       30 200       Livelient       00 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Very detti       30 200       Standard       Very detti       30 200       Standard </td <td></td> <td></td> <td></td> <td>Compact</td> <td>10 - 3</td> <td>30</td> <td>Firm</td> <td></td> <td></td> <td></td> <td>2</td> <td>5 - 50</td> <td>Fair</td> <td></td> <td>50</td> <td>- 75 %</td> <td>Moder</td> <td></td> <td></td> <td>200 -</td> <td>600 m</td>				Compact	10 - 3	30	Firm				2	5 - 50	Fair		50	- 75 %	Moder			200 -	600 m
Toppol.     Description of Soll.S     OB     Image: Solution of Soll.S     OB     Image: Solution of Soll.S     Image: Solution of Soll.S <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th>																					
Image: Second second		Cor													1		Wide			> 6	5000 m
Image: Construction of the application			STRATIO	GRAPHY				S	AN	<b>IP</b>	LES	3									
Image: Construction of the application		_								(				Ъ М	S : se	dimenton	netry	\(\no: Nc (d	yn. per		
Image: Construction of the application	Ê	E.	<u>e</u>		_			PLE	2	(%		Sta	andard	NFL NFL	C :co W :wa	iter conte				ed	S
Image: Construction of the application	5)5 1 1 1	NO	트 DESCF	RIPTION OF SOILS	BOL	STATE	TYPE N	SUB - SAM	BE	RECOVERN	å	pen	etration	R I	W <sub>1</sub> :liq	uid limit		🔶 : Su in	act		A R
Image: Construction of the application	퓐딘	LAT TAT		AND ROCK	λN				CAL		2			ATE	Dr : sp	permeability					KEM.
1     CA: chemical analyses		1										BLOW	/S/150mm		f'c : co	"c : compressive str. DM: organic matter				•	
Beddsh-brown, SAND with some fine gravel.     - preserve of cobbles and boulders       - moist     - moist       GA     - moist       Bork grey, sithy SAND.     - moist	_	0.0	00		~~~	;									CA : ch	emical an	alyses		<u> </u>		
enter remotes (silt and clay) and traces of gravel. - moist - mo		\ -0.0	05 / Deddieb brees	n, SAND with some fine		ł															
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1     -1.75     Grey, SAND with some silt and traces of gravel.       - moist       3     -0       - 3.25     Dark grey, silty SAND.       - 3.30     -0       - moist					8	łV															
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F. Beaudry-Polvin, deo	veneral	remai	iks:													Veri	iea by :		udry P	otvin -	-

Q	O Stantec       TEST PIT REPORT         Project:       James Bay Lithium Pegmatite Project -       Location :       UTM, zone 18       Borehole :       PBS-22														PORT								
Proj	Project: James Bay Lithium Pegmatite Project - Potential Borrow Source Assessment						n :		TM, z 57749		18					Boreho Page :		PB	S-22-1				
Proj	Project No.: <b>121622255.220.700</b>								5789464.3								ate :	20'	1 of 1 19-08-01				
1	Client: Galaxy Lithium (Canada) Inc.						f borehol					3				Inspec			M. Verpaelst				
	Site: Vicinity of roadstop KM381 - James Bay Road					quipn asing	nent : s :		and s mm	nove	:1					Depth			0.70 m				
Figu	Figure:								mm								ion :		m				
		SAMPL		QUALITATIVE TERMINOL				NTITA	TIVE T	ERMI				SYMBOL	_			<u>GROUNDWATER</u>					
SS   C	5		uous sampling	Silt 0.002 - 0		m	Traces Some				10	< 10 % - 20 %	(AST	dard penet M D 1586)					Depth				
D A	5	Auger	nd rock core	Gravel 5	8 - 5 m - 80 m	m	Adjective and (ex: a	and gra			;	- 35 % > 35 %	(BNC	amic cone p 2 2501-145	)		Reading Reading		m m				
T\ S1	-	Shelby			200 m 200 m		Main wo	rd Dominant fracti				action	RQD Rock	Quality De	esignation	(%)	Remark	Remarks :					
M	A		I sample																				
	$\leq$	SAMPL Remou	<u>.E STATE</u> Ilded	MECHANIC COMPACTION IN	CHAR DEX "N		CONSIST		i	Cu	OR SL	ı (kPa)	QUALIFICA	<u>K QUALITY</u> ATIVE	DESIGNA	rion RQD	Very tig	JOINTS SPACING	< 20 mm				
			(thin wall sampler)	Very loose Loose	0 - 4 - 1	4	Very soft Soft					< 12 2 - 25	Very poor Poor			< 25 % - 50 %	Tight Close	2	0 - 60 mm - 200 mm				
		Lost	,	Compact Dense	10 - 3 30 - 5	30	Firm Stiff				2	25 - 50 ) - 100	Fair Good		50	- 75 % - 90 %		ately spaced 200	- 600 mm 2000 mm				
		Core (c	liamond rock core)	Very dense	> 5	50	Very stiff				100	) - 200	Excellent			- 90 %	Very sp	aced 2000 -	6000 mm				
			STDATIC				Hard		• • •			> 200			1		Wide		6000 mm				
			STRATIG	RAPHI				3	SAMPLES					- 2	GA : gra	ain size ar		ESTS $\times$ : N (standard pen.)					
		1						щ		(%				WATER LEVEL / WATER INFLOW	S : sec	dimenton nsolidatio	netry	▽ : Nc (dyn. pen.) ■ : Cu intact					
DEPTH (m)	(Ħ	_EVATION (m) / DEPTH (m)			Р	ш	å	MPI	н	RECOVERY (%)	a		andard etration	Ш Ц	W : water content W, : liquid limit			□ : Cu remoulded ◆ : Su intact	REMARKS				
E	DEPTH (ft)	PTH PTH		PTION OF SOILS ND ROCK	SYMBOL	STATE	TYPE N°	SUB - SAMPLE	CALIBER	N N	- RQD	test		TER	W <sub>p</sub> :pla	astic limit ecific grav	ritv	$\diamond$ : Su remoulded	MAF				
	В	ЪВ			S	Ś	F		5	С Ш	2	BLOW	/S/150mm	A W A	k :pe	rmeabilit <sup>,</sup> mpressive	y	w <sub>₽</sub> w w <sub>L</sub>	RE				
		법 0.00						S		R				<b>₹</b> ←		ganic mat emical an		20 40 60 80					
_		0.00	Topsoil.	AND with some															_				
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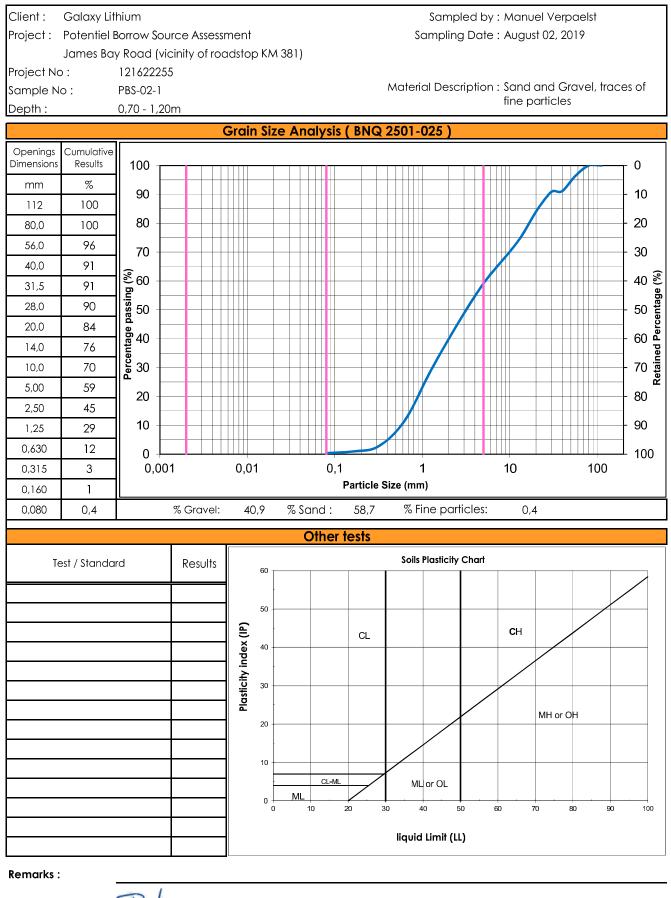
C	Sta	antec															Т	ΈS	ΤP	'IT F	REP	PORT		
Pro	ect:	Jame Poter		Location : UTM, zone 18 X : 358752.8											Borehole : PBS-2									
Pro	Project No.: <b>121622255.220.700</b>						Y : 5789021.1									Page Start o		2019	1 of 1 9-08-03					
	Client: Galaxy Lithium (Canada) Inc.						Type of borehole : Manual sampling Equipment : Hand shovel									Inspec			M. Verpaelst					
	Site: Vicinity of roadstop KM381 - James Bay Road						s:		mm							Depth : 2.0 Elevation :								
		SAMP	LE TYPE	QUALITATIVE TERMINOL		orer :		NTITA	mm TIVE T	ERMI	NOLC	GY		SYMBOL	s			GR		WATE	2	m		
s		Split s			002 m		Traces Some					< 10 % - 20 %		dard penet M D 1586)		lue			Dat			epth		
	с		nd rock core	Sand 0.08	- 5 m - 80 m	m	Adjective and (ex:		avel)		20	- 35 % > 35 %	Nc Dyna	amic cone p 2 2501-145	oenetratio	on value	Reading Reading					m m		
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	$\leq$	SAMPI Remou	L <u>E STATE</u> Jlded	COMPACTION INE	<u>CHAR</u> DEX "N		CONSIST		5	Cu	OR SI	u (kPa)	QUALIFIC	<u>K QUALITY</u> ATIVE	DESIGNA	<u>TION</u> RQD	Very tig		OINTS	S SPACI		< 20 mm		
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## **APPENDIX E**

Laboratory Testing Reports

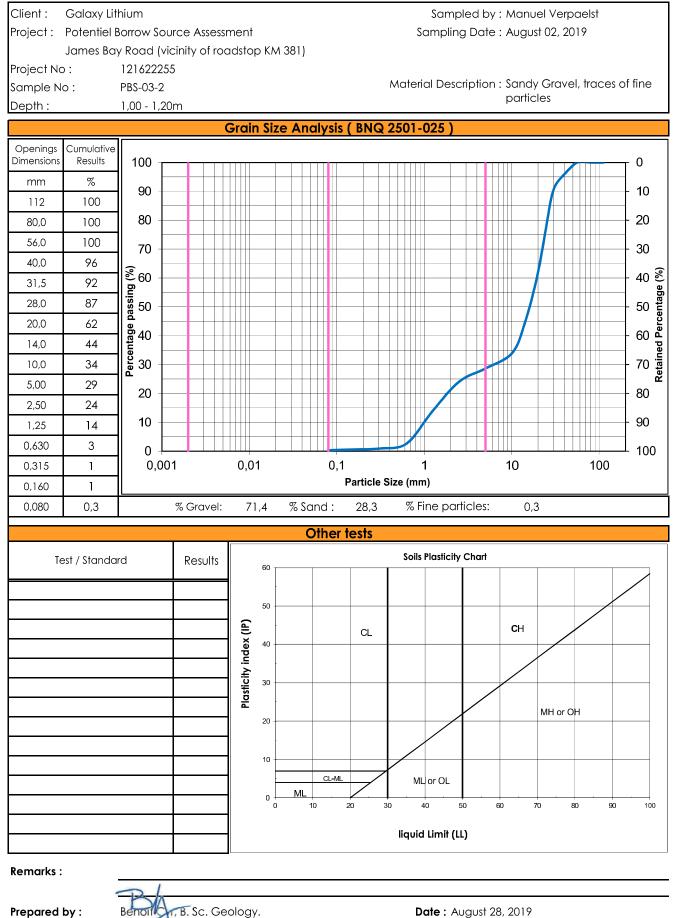




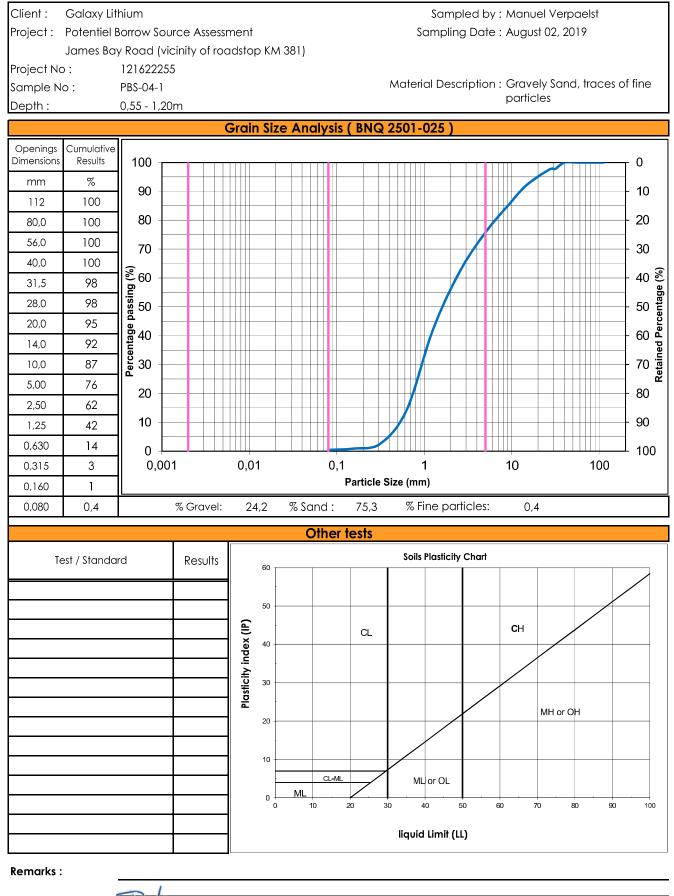


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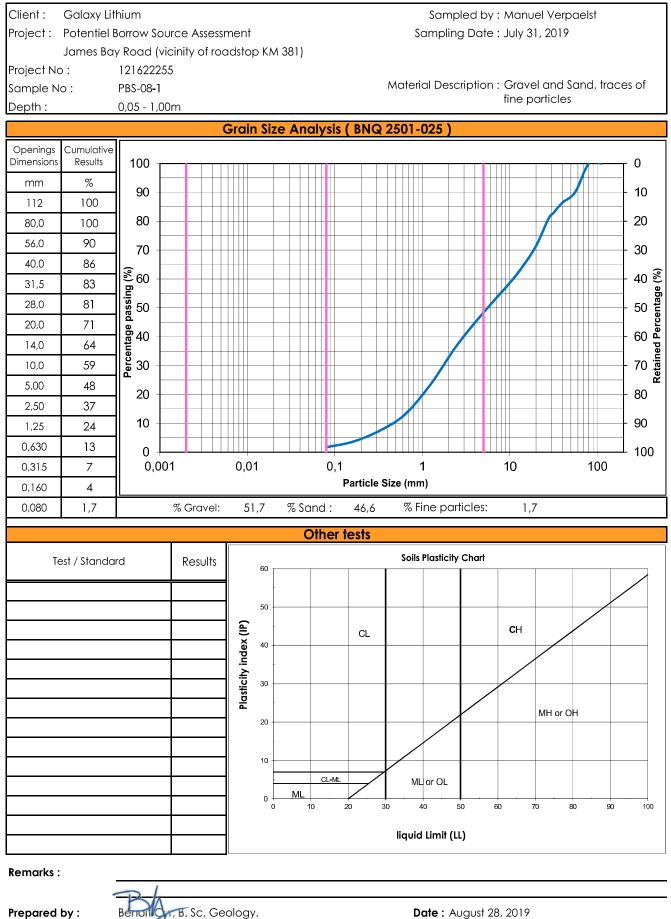






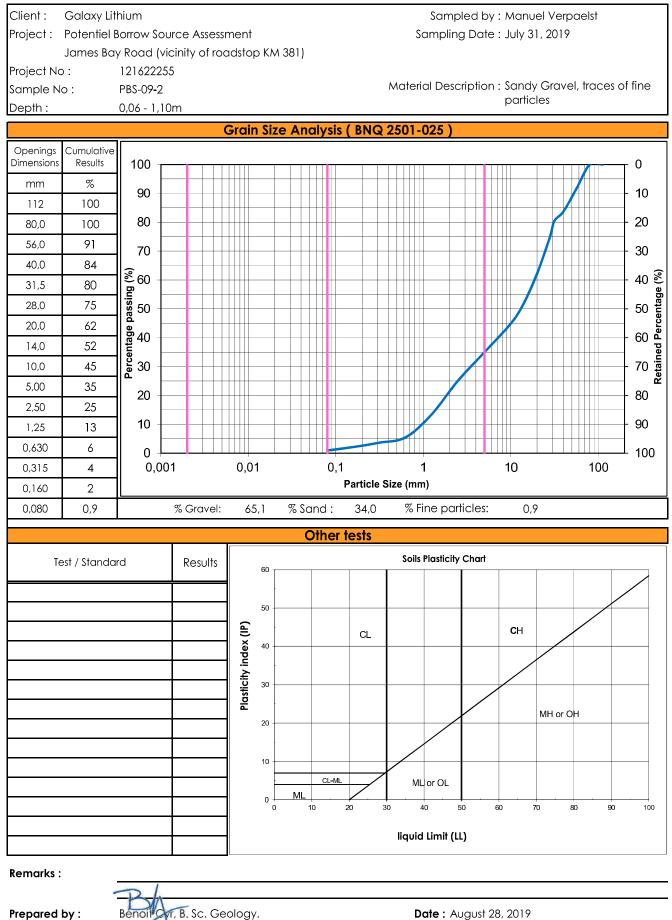
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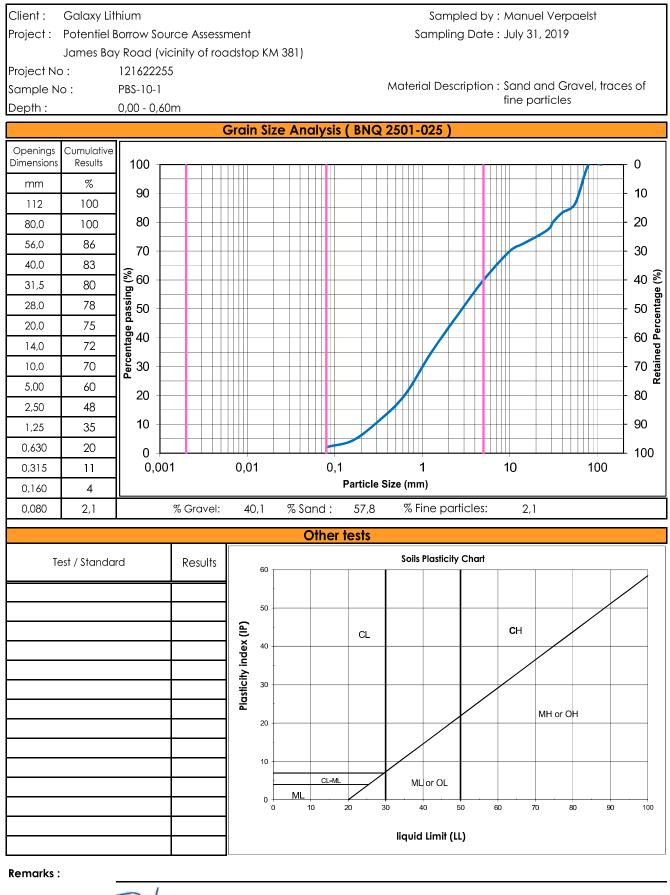




## LABORATORY TESTING REPORT





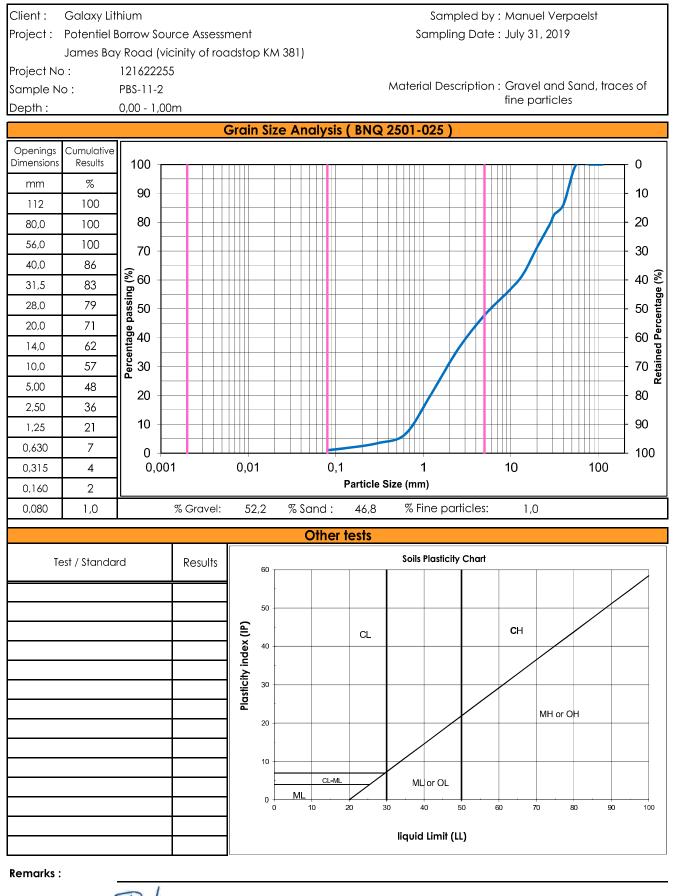


Date: August 28, 2019

Prepared by :

Sc. Geology.





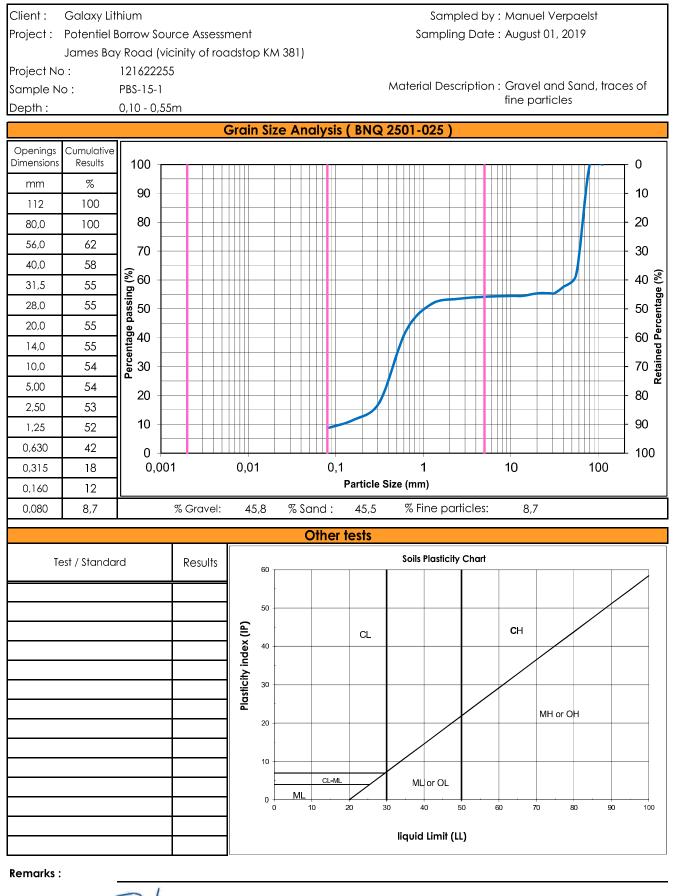
Date: August 28, 2019

Prepared by :

B. Sc. Geology.

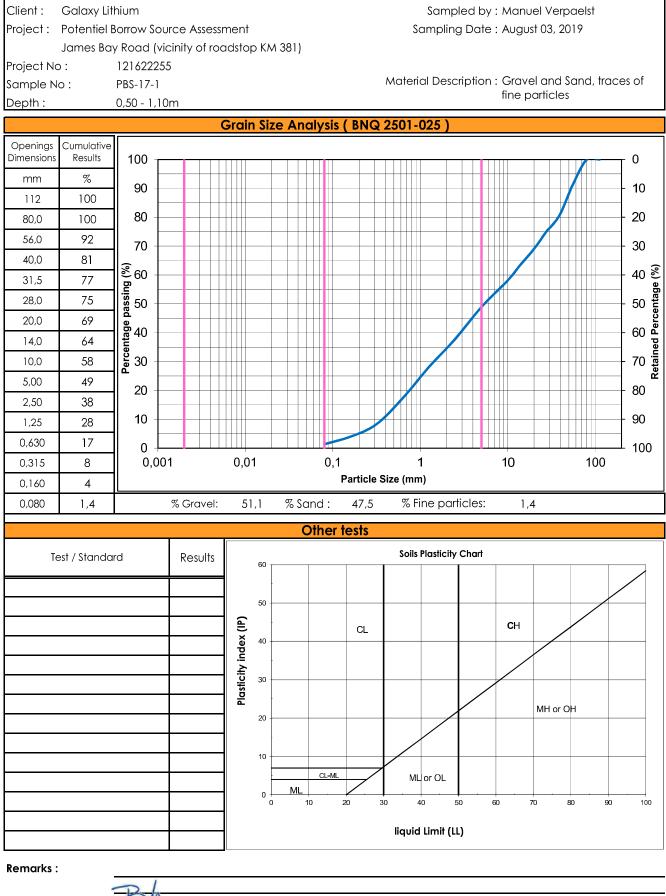
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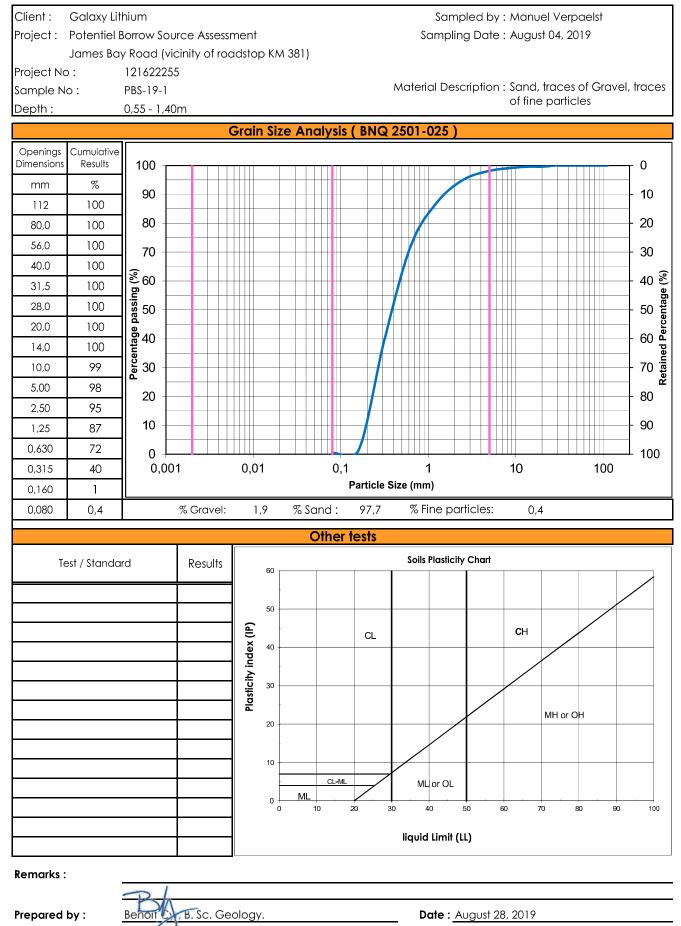
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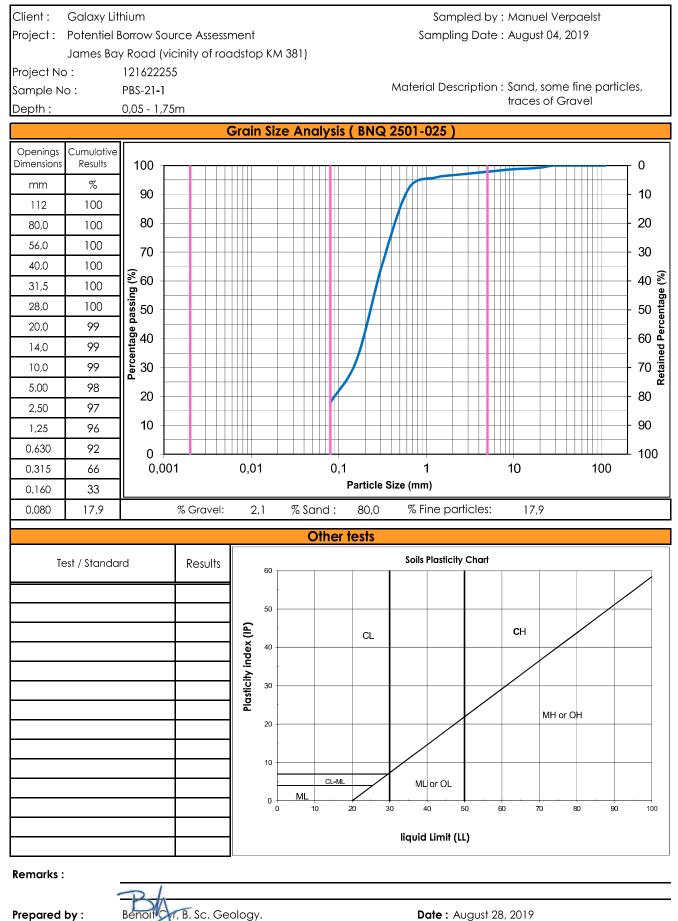


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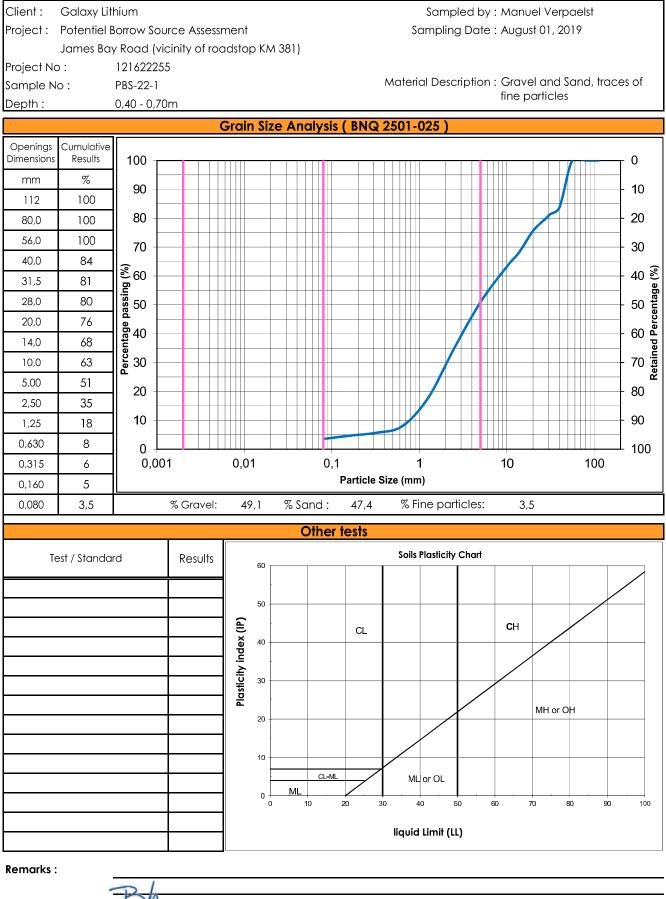


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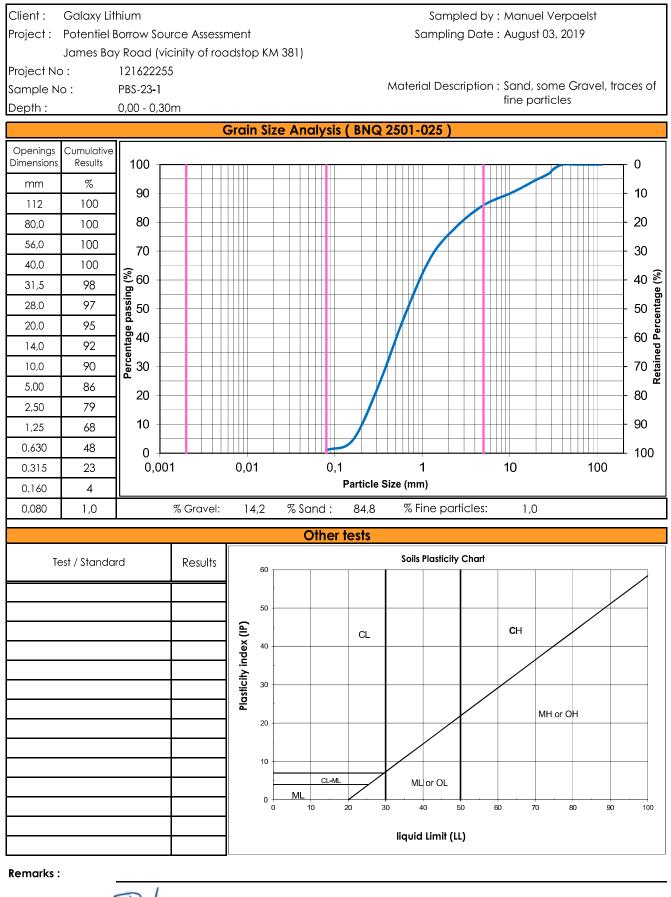
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Sc. Geology.

## **APPENDIX F** Photographic Album





Photo 1 : Test pit PBS-02-1



Photo 2 : Test pit PBS-03-1





Photo 3 : Test pit PBS-03-2



Photo 4 : Test pit PBS-04-1





Photo 5 : Test pit PBS-07-1



Photo 6 : Test pit PBS-08-1





Photo 7 : Test pit PBS-08-2



Photo 8 : Test pit PBS-09-1





Photo 9 : Test pit PBS-09-2



Photo 10 : Test pit PBS-10-1





Photo 11 : Test pit PBS-11-1



Photo 12 : Test pit PBS-11-2





Photo 13 : Test pit PBS-11-3



Photo 14 : Test pit PBS-11-4





Photo 15 : Test pit PBS-13-1



Photo 16 : Test pit PBS-15-1





Photo 17 : Test pit PBS-15-2



Photo 18 : Test pit PBS-16-1





Photo 19 : Test pit PBS-17-1



Photo 20 : Test pit PBS-18-1





Photo 21 : Test pit PBS-19-1



Photo 22 : Test pit PBS-19-2





Photo 23 : Test pit PBS-19-3



Photo 24 : Test pit PBS-21-1





Photo 25 : Test pit PBS-22-1



Photo 26 : Test pit PBS-23-1

